DRAINAGE CALCULATIONS, HYDRAULICS & HYDROLOGY REPORT

PROPOSED WAREHOUSES 69 FITCH HILL ROAD UNCASVILLE, CT

APRIL 2023



GREEN SITE DESIGN

69 FITCH HILL R.OAD MONTVILLE, CT

The site was previously developed as a gravel excavation site, with areas of previous excavation, stockpiles, and cleared land. The site is proposed to be developed with three new warehouse buildings. Building 1 will be for A&B Excavation. Buildings 2 and 3 will be rental space. Green Site Design is providing the design and calculations for the stabilization of the site.

PROPOSED HYDRAULICS

There will be two main on-site drainage areas for the proposed development. Please see the attached plans entitled Drainage Plan – Drainage Map.

Drainage area 1 will handle runoff from the Building 1 area, A&B Excavation, and northern halves of Buildings 2&3. The runoff from these areas will flow to the wetlands, after flowing thru Stormwater basin 1, resulting in a decrease in peak flows to the wetlands system.

Drainage area 2 will handle runoff from the southern halves of Buildings 2&3 and their associated areas. The runoff from these areas will flow to the wetlands, after flowing thru Stormwater Basin 2, resulting in a decrease in peak flows to the existing wetlands system.

The soils on the site are extremely well drained gravel and sand. Numerous test holes were done on the site, and showed the soils and groundwater levels to be very consistent. The rainwater during most storm events will infiltrate into the ground with little runoff.

Both the existing and the proposed conditions for the development site have been analyzed for the 2-year, 10-year, 25-year, and 100-year design storms using the TR-55

SCS modelling program. The following is the summary table for the 2-year, 10-year, 25-year, and 100-year design storms showing first the existing conditions and proposed conditions, after passing thru the proposed stormwater basins. The calculations show that there will be a decrease in runoff leaving the site. The following are the results of the computer model.

Drainage Area 1

| Diamage Mea 1 | | | | |
|-----------------|-----------|-----------|----------------|-----------------|
| | 2-year | 10-year | <u>25-year</u> | <u>100-year</u> |
| Existing | 0.265 cfs | 1.847 cfs | 3.208 cfs | 5.659 cfs |
| Proposed | 0.000 cfs | 0.000 cfs | 0.000 cfs | 5.325 cfs |
| | | | | |
| Drainage Area 2 | | | | |
| | 2-year | 10-year | 25-year | <u>100-year</u> |
| Existing | 0.381 cfs | 2.339 cfs | 4.056 cfs | 7.095 cfs |
| Proposed | 0.000 cfs | 0.000 cfs | 0.000 cfs | 6.325 cfs |
| | | | | |

CT GUIDELINES FOR SOIL EROSION & SEDIMENTATION CONTROL

The 2002 CT Guidelines for Soil Erosion & Sedimentation Control applies to the construction phase of the project. A detailed erosion and sediment control plan has been provided in the site development plans. The proposed Stormwater Basins have been designed to function as sedimentation traps during stabilization, and then as stormwater basins to provide permanent water quality treatment, prior to entering the existing off-site drainage systems, for the life of the facility.

Drainage Area 1, Temporary Sediment Trap 1

The first calculation required by the Guidelines is for the sediment storage volume (SSV). The sediment storage volume is the calculation for one year of predicted

sediment load. The calculations for a Temporary Sediment Trap show that the sediment storage volume required is <u>14,834 CF</u>:

The second calculation required by the Guidelines is for wet storage volume (WSV). The wet storage volume is the volume in the basin that is located below the riprap for the level spreader outlet of the basin (elevation 38.5). The volume of the wet storage is required to be half of the required SSV. The required wet storage volume is 14,834 CF/2 = 7,417 CF. The required dry storage volume, located above the bottom of the riprap of the level spreader outlet of the basin (elevation 38.5), is 7,417 CF.

The total storage volume required is the dry storage volume plus the wet storage volume, which is a total of 14,834 CF.

The combined volume required for the Sedimentation Basin as follows:

| 7,417 CF of Wet Storage Volume | 15,320 CF Provided |
|------------------------------------|--------------------------|
| 7,417 CF of Dry Storage Volume | 12,020 CF Provided |
| 14,834 CF of Total Volume Required | 27,340 CF Total Provided |

Drainage Area 2 Temporary Sediment Trap 2

The first calculation required by the Guidelines is for the sediment storage volume (SSV). The sediment storage volume is the calculation for one year of predicted sediment load. The calculations for a Temporary Sediment Trap show that the sediment storage volume required is 9,045CF:

SSV = A(134CY/Acre)

A = 2.5 ACRES

SSV = 335 CY = 9.045 CF

The second calculation required by the Guidelines is for wet storage volume (WSV). The wet storage volume is the volume in the basin that is located below the riprap for the level spreader outlet of the basin (elevation 38.5). The volume of the wet storage is required to be half of the required SSV. The required wet storage volume is 9,045 CF/2 = 4,523 CF. The required dry storage volume, located above the bottom of the riprap of the level spreader outlet of the basin (elevation 38.5), is 4,523 CF.

The total storage volume required is the dry storage volume plus the wet storage volume, which is a total of 9,045 CF.

The combined volume required for the Sedimentation Basin as follows:

4,523 CF of Wet Storage Volume

10,426 CF Provided

4,523 CF of Dry Storage Volume

8,686 CF Provided

9.045 CF of Total Volume Required

19,112 CF Total Provided

CONNECTICUT STORMWATER QUALITY MANUAL

The Stormwater Management System, consisting of two and Water Quality Basins, have been designed to function as permanent water quality treatment for the life of the facility. The Connecticut 2004 Stormwater Quality Manual (Manual) applies to the post construction phase, for the operation of the facility.

Drainage Area 1, Water Quality Basin 1

The Stormwater Management System meets the criteria of the Connecticut Stormwater Quality Manual for a Water Quality Basin. The calculations show that a Water Quality Volume (WQV) of 6,621 CF is required:

As the calculations show that there will be no stormwater leaving the proposed stormwater management system (water quality basin), up to and including the 25 year storm event, the anticipated pollutant removal rate is 100%.

Drainage Area 2, Water Quality Basin 2

The Stormwater Management System meets the criteria of the Connecticut Stormwater Quality Manual for a Water Quality Basin. The calculations show that a Water Quality Volume (WQV) of 6,142 CF is required:

As the calculations show that there will be no stormwater leaving the proposed stormwater management system (water quality basin), up to and including the 25 year storm event, the anticipated pollutant removal rate is 100%.

The University of New Hampshire's Stormwater Center in Durham New Hampshire indicates that typical Phosphorus load export rate from this type of development (commercial/industrial) will be 1.78 lbs/acre/year. For Nitrogen that value will be 15 lbs/acre/year.

(https://www.unh.edu/unhsc/sites/default/files/media/ms4_permit_nomographs_shee t_final_2020.pdf) The University of New Hampshire's research reveals that efficiency removal for typical pollutants of concern such as TSS, N, P, and zinc is directly tied to the volume of stormwater that is held and infiltrated. The research reveals that if a 2 inch depth of runoff from a site's impervious surface is held and infiltrated by a given BMP, the reduction in these pollutants is 99-100%. On this site, both Stormwater basin #1 and Stormwater basin #2 will contain up to and including the 25 year storm event. Thus, CLA believes that pollutant removal rates for pollutants of concern will be greater than 99% and there will be no increase in releases of pollutants to the wetlands system.

Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Thursday, Apr 27, 2023

Pond No. 2 - Stormwater Basin 2

Pond Data

Contours - User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 36.00 ft

Stage / Storage Table

| Stage (ft) | Elevation (ft) | Contour area (sqft) | Incr. Storage (cuft) | Total storage (cuft) |
|------------|----------------|---------------------|----------------------|----------------------|
| 0.00 | 36.00 | 3,114 | 0 | 0 |
| 1.00 | 37.00 | 3,925 | 3,511 | 3,511 |
| 2.00 | 38.00 | 4,735 | 4,323 | 7,835 |
| 3.00 | 39.00 | 5,645 | 5,183 | 13,017 |
| 4.00 | 40.00 | 6,556 | 6,094 | 19,112 |

Culvert / Orifice Structures Weir Structures [PrfRsr] [C] [D] [A] [B] [C] [A] [B] = 0.000.00 0.00 0.00 = 10.000.00 0.00 0.00 Rise (in) Crest Len (ft) = 38.50 = 0.000.00 0.00 0.00 Span (in) 0.00 Crest El. (ft) 0.00 0.00 Weir Coeff. = 3.333.33 3.33 No. Barrels **=** 0 0 3.33 Invert El. (ft) = 0.000.00 0.00 0.00 Weir Type = Broad Length (ft) = 0.000.00 0.00 0.00 Multi-Stage = No No No No = 0.000.00 0.00 Slope (%) n/a .013 N-Value = .013.013 n/a Orifice Coeff. = 0.600.60 0.60 0.60 Exfil.(in/hr) = 15.000 (by Contour) = 0.00Multi-Stage = n/aNo No No TW Elev. (ft)

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

| • | _ | | | | | | | | | | | |
|--------------|--|---|---|--|---|--|--|--|---|---|---|---|
| Storage cuft | Elevation ft | CIv A cfs | Clv B cfs | CIv C cfs | PrfRsr cfs | Wr A cfs | Wr B cfs | Wr C cfs | Wr D cfs | Exfil cfs | User cfs | Total cfs |
| 0 | 36.00 | | | | | 0.00 | | | | 0.000 | | 0.000 |
| 3,511 | 37.00 | | | | | 0.00 | | | | 1.363 | | 1.363 |
| 7,835 | 38.00 | | | | | 0.00 | | | | 1.644 | | 1.644 |
| 13,017 | 39.00 | | | | | 11.77 | | | | 1.960 | | 13.73 |
| 19,112 | 40.00 | | | | | 61.18 | | | | 2.276 | | 63.45 |
| | Storage cuft 0 3,511 7,835 13,017 | Storage cuft ft 0 36.00 3,511 37.00 7,835 38.00 13,017 39.00 | Storage cuft Elevation ft CIv A cfs 0 36.00 3,511 37.00 7,835 38.00 13,017 39.00 | Storage cuft Elevation ft CIv A cfs CIv B cfs 0 36.00 3,511 37.00 7,835 38.00 13,017 39.00 | Storage cuft Elevation ft Civ A cfs Clv B cfs Clv C cfs 0 36.00 3,511 37.00 7,835 38.00 13,017 39.00 | Storage cuft Elevation ft Clv A cfs Clv B cfs Clv C cfs PrfRsr cfs 0 36.00 3,511 37.00 7,835 38.00 13,017 39.00 | Storage cuft Elevation ft Clv A cfs Clv B cfs Clv C cfs PrfRsr cfs Wr A cfs 0 36.00 0.00 | Storage cuft Elevation ft Clv A cfs Clv B cfs Clv C cfs PrfRsr cfs Wr A cfs Wr B cfs 0 36.00 0.00 3,511 37.00 0.00 7,835 38.00 0.00 13,017 39.00 11.77 | Storage cuft Elevation ft Clv A cfs Clv B cfs Clv C cfs PrfRsr cfs Wr A cfs Wr B cfs Wr C cfs 0 36.00 0.00 | Storage cuft Elevation ft Clv A cfs Clv B cfs Clv C cfs PrfRsr cfs Wr A cfs Wr B cfs Wr C cfs Wr D cfs 0 36.00 0.00 3,511 37.00 0.00 7,835 38.00 0.00 13,017 39.00 | Storage cuft Elevation ft Clv A cfs Clv B cfs Clv C cfs PrfRsr cfs Wr A cfs Wr B cfs Wr C cfs Wr D cfs Exfil cfs 0 36.00 0.00 0.00 3,511 37.00 0.00 1.363 7,835 38.00 0.00 1.644 13,017 39.00 11.77 1.960 | Storage cuft Elevation ft Clv A cfs Clv B cfs Clv C cfs PrfRsr cfs Wr A cfs Wr B cfs Wr C cfs Wr D cfs Exfil User cfs 0 36.00 0.00 0.000 0.000 1.363 7,835 38.00 0.00 1.644 13,017 39.00 11.77 1.960 |

Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Thursday, Apr 27, 2023

Pond No. 1 - Stormwater Basin 1

Pond Data

N-Value

Orifice Coeff.

Multi-Stage

Contours - User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 36.00 ft

Stage / Storage Table

| Stage (ft) | Elevation (ft) | Contour area (sqft) | Incr. Storage (cuft) | Total storage (cuft) |
|------------|----------------|---------------------|----------------------|----------------------|
| 0.00 | 36.00 | 4,894 | 0 | 0 |
| 1.00 | 37.00 | 5,840 | 5,360 | 5,360 |
| 2.00 | 38.00 | 6,786 | 6,306 | 11,666 |
| 3.00 | 39.00 | 7,844 | 7,308 | 18,974 |
| 4.00 | 40.00 | 8,901 | 8,366 | 27,340 |

.013

0.60

No

n/a

0.60

No

.013

0.60

No

Culvert / Orifice Structures Weir Structures [A] [B] [C] [PrfRsr] [A] [C] [D] [B] Rise (in) = 0.000.00 0.00 0.00 Crest Len (ft) = 10.000.00 0.00 0.00 = 0.000.00 0.00 0.00 Crest El. (ft) 0.00 Span (in) = 38.500.00 0.00 No. Barrels Weir Coeff. = 2.603.33 = 1 3.33 3.33 Invert El. (ft) = 0.000.00 0.00 0.00 Weir Type = Broad Length (ft) = 0.000.00 0.00 0.00 Multi-Stage = No No No No 0.00 0.00 = 0.00n/a Slope (%)

Exfil.(in/hr)

TW Elev. (ft)

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

= 0.00

= 12.000 (by Contour)

Stage / Storage / Discharge Table

= .013

= 0.60

= n/a

| • | _ | _ | | | | | | | | | | | |
|-------------|-----------------|-----------------|--------------|--------------|--------------|---------------|-------------|-------------|-------------|-------------|--------------|-------------|--------------|
| Stage ft | Storage cuft | Elevation ft | CIv A cfs | Civ B cfs | Clv C cfs | PrfRsr cfs | Wr A cfs | Wr B cfs | Wr C cfs | Wr D cfs | Exfil cfs | User cfs | Total cfs |
| 0.00 | 0 | 36.00 | | *** | | | 0.00 | | | | 0.000 | | 0.000 |
| 1.00 | 5,360 | 37.00 | *** | *** | ~~~ | | 0.00 | B-0 B | | | 1.622 | | 1.622 |
| 2.00 | 11,666 | 38.00 | | | | | 0.00 | 800 | | | 1.885 | | 1.885 |
| 3.00 | 18,974 | 39.00 | | | | | 9.19 | | | | 2.179 | | 11.37 |
| 4.00 | 27,340 | 40.00 | | | | | 47.77 | | | | 2.472 | | 50.24 |
| | | | | | | | | | | | | | |

Hydrograph Summary Report
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

| jyd. No. | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to peak (min) | Hyd. volume (cuft) | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph description |
|-------------|--------------------------------|--|---------------------------|--|--------------------------|--|------------------------------|--|-------------------------------|
| 1 | SCS Runoff | 0.265 | 1 | 737 | 2,133 | | | | Existing Conditions - Areas 1 |
| 2 | SCS Runoff | 3.444 | 1 | 724 | 11,499 | | ****** | | Proposed Conditions - Area 1 |
| 3 | Reservoir | 0.000 | 1 | 744 | 0 | 2 | 36.56 | 2,979 | Stormwater Basin 1 |
| 4 | SCS Runoff | 0.381 | 2 | 730 | 2,718 | ***** | | | Existing Conditions - Area 2 |
| 5 | SCS Runoff | 2.633 | 2 | 726 | 8,856 | | | | Proposed Conditions - Area 2 |
| 6 | Reservoir | 0.000 | 2 | 728 | 0 | 5 | 36.71 | 2,490 | Stormwater Basin 2 |
| | | | | | | | | | |
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|) | | | | | | | | 1 | |
| | A&B Excavation TR55.gpw | | | | Return F | Return Period: 2 Year | | | Apr 27, 2023 |

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Thursday, Apr 27, 2023

Hyd. No. 1

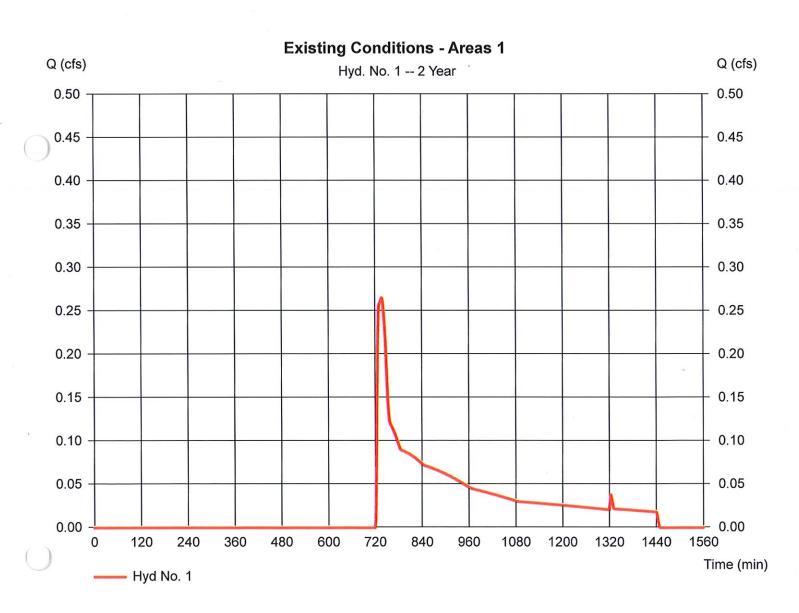
Existing Conditions - Areas 1

Hydrograph type = SCS Runoff Storm frequency = 2 yrsTime interval = 1 min Drainage area = 1.900 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 3.36 inStorm duration = 24 hrs

Peak discharge = 0.265 cfs
Time to peak = 737 min
Hyd. volume = 2,133 cuft
Curve number = 55*
Hydraulic length = 0 ft
Time of conc. (Tc) = 5.31 min

Distribution = Type III Shape factor = 484

^{*} Composite (Area/CN) = [(4.100 x 55)] / 1.900



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Thursday, Apr 27, 2023

Hyd. No. 2

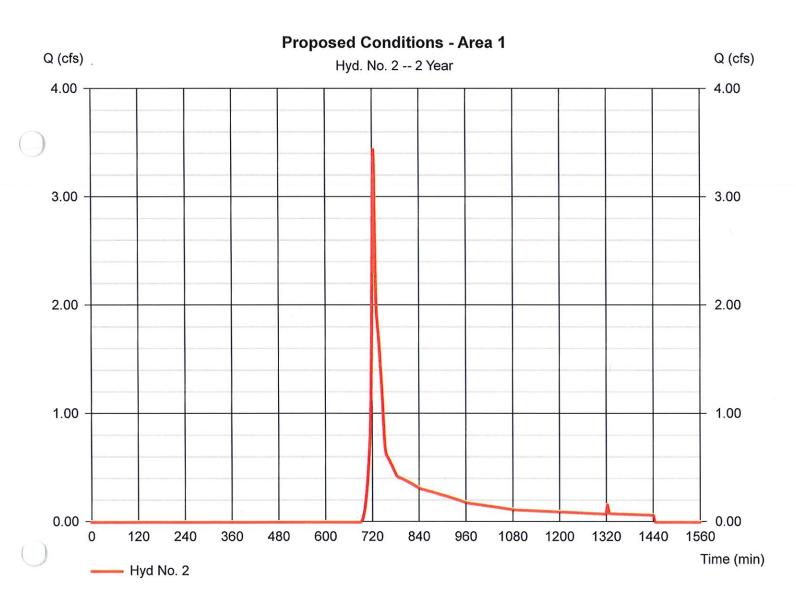
Proposed Conditions - Area 1

= SCS Runoff Hydrograph type Storm frequency = 2 yrsTime interval = 1 min Drainage area = 4.100 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 3.36 inStorm duration = 24 hrs

Peak discharge = 3.444 cfs
Time to peak = 724 min
Hyd. volume = 11,499 cuft

Curve number = 67*
Hydraulic length = 0 ft
Time of conc. (Tc) = 4.50 min
Distribution = Type III
Shape factor = 484

^{*} Composite (Area/CN) = [(1.800 x 83) + (2.300 x 55)] / 4.100



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Thursday, Apr 27, 2023

Hyd. No. 3

Stormwater Basin 1

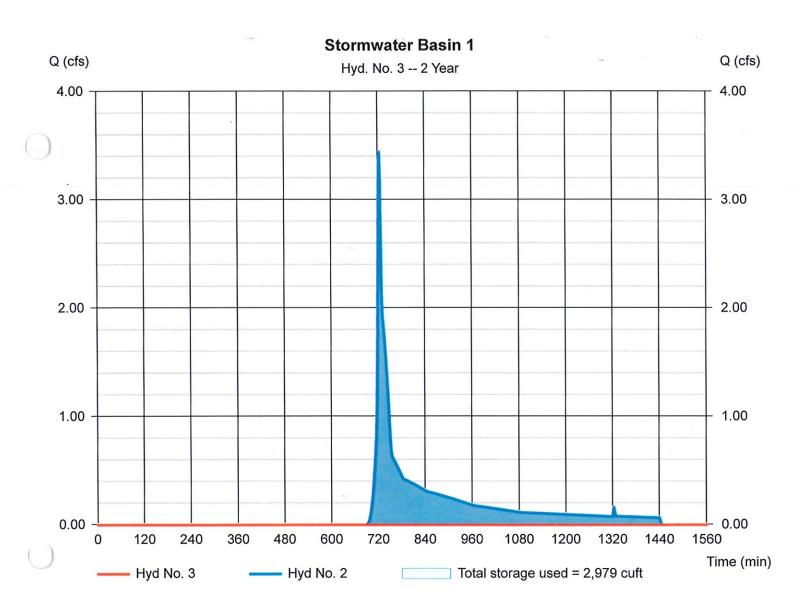
Hydrograph type = Reservoir Storm frequency = 2 yrs Time interval = 1 min

Inflow hyd. No. = 2 - Proposed Conditions - Area 1
Reservoir name = Stormwater Basin 1

Peak discharge = 0.000 cfs Time to peak = 744 min

Hyd. volume = 0 cuft
Max. Elevation = 36.56 ft

Max. Storage = 2,979 cuft



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Thursday, Apr 27, 2023

Hyd. No. 4

Existing Conditions - Area 2

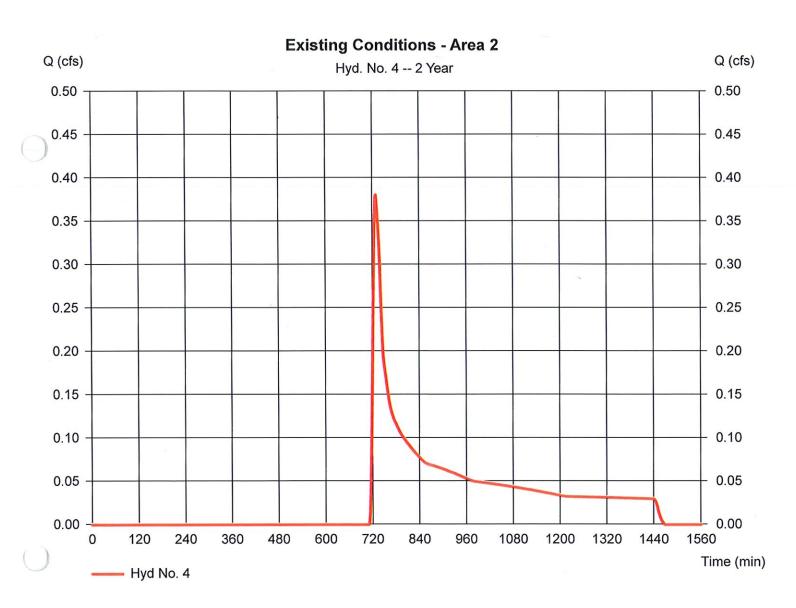
Hydrograph type = SCS Runoff
Storm frequency = 2 yrs
Time interval = 2 min
Drainage area = 2.500 ac
Basin Slope = 0.0 %
Tc method = TR55

Tc method = TR55 Total precip. = 3.36 in Storm duration = 24 hrs Peak discharge = 0.381 cfs Time to peak = 730 min Hyd. volume = 2,718 cuft

Curve number = 55^* Hydraulic length = 0 ft

Time of conc. (Tc) = 16.90 min
Distribution = Type II
Shape factor = 484

^{*} Composite (Area/CN) = [(2.500 x 55)] / 2.500



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Thursday, Apr 27, 2023

Hyd. No. 5

Storm duration

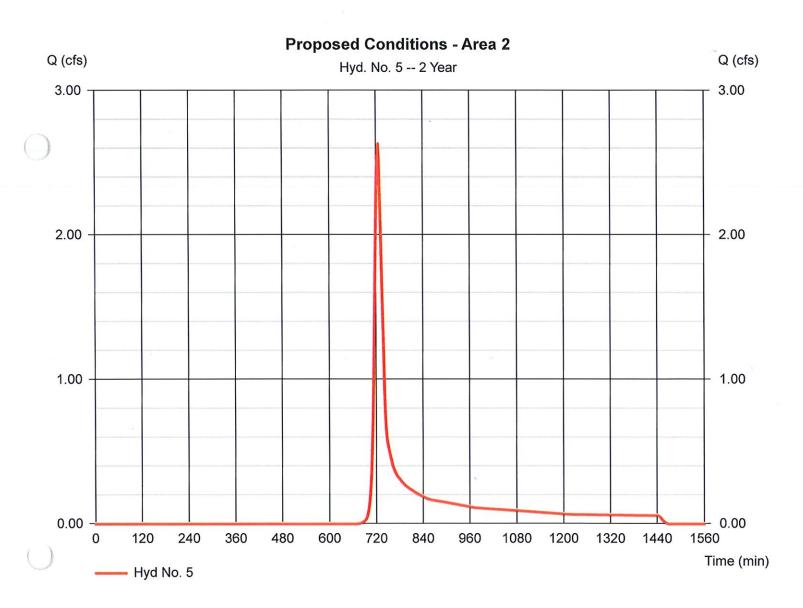
Proposed Conditions - Area 2

Hydrograph type = SCS Runoff
Storm frequency = 2 yrs
Time interval = 2 min
Drainage area = 2.500 ac
Basin Slope = 0.0 %
Tc method = TR55
Total precip. = 3.36 in

Peak discharge = 2.633 cfs Time to peak = 726 min Hyd. volume = 8,856 cuft Curve number = 71*

Hydraulic length = 0 ft
Time of conc. (Tc) = 16.49 min
Distribution = Type II
Shape factor = 484

= 24 hrs



^{*} Composite (Area/CN) = [(1.400 x 83) + (1.100 x 55)] / 2.500

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Thursday, Apr 27, 2023

Hyd. No. 6

Stormwater Basin 2

Hydrograph type = Reservoir Storm frequency = 2 yrs Time interval = 2 min

Inflow hyd. No. = 5 - P

Reservoir name = Stormwater Basin 2

= 5 - Proposed Conditions - Area 2

Peak discharge

= 0.000 cfs

Time to peak Hyd. volume

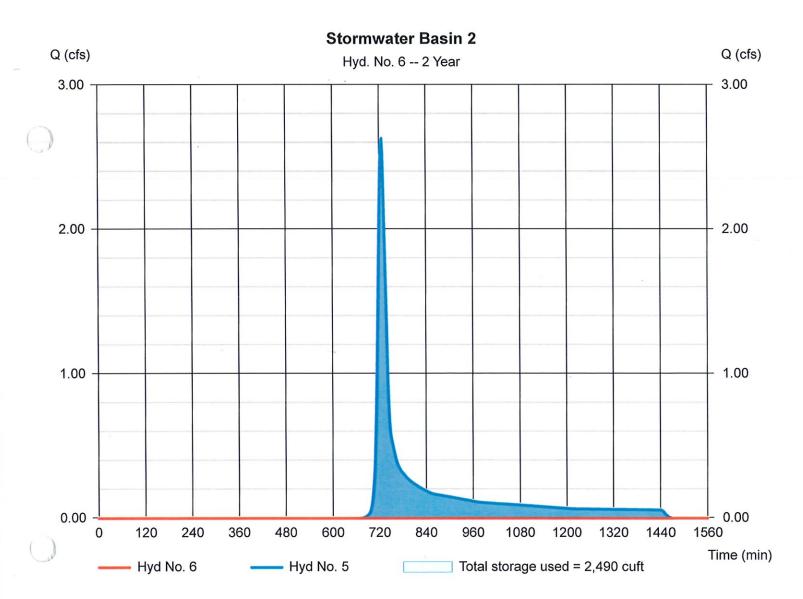
= 728 min

Max. Elevation

= 0 cuft = 36.71 ft

Max. Storage

= 2,490 cuft



Hydrograph Summary Report
Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

| /d. No. | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to peak (min) | Hyd. volume (cuft) | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph description |
|--|--------------------------------|-----------------------|---------------------------|--------------------------|--|--|--|-------------------------------|-------------------------------|
| 1 | SCS Runoff | 1.847 | 1 | 726 | 7,041 | | | | Existing Conditions - Areas 1 |
| 2 | SCS Runoff | 9.052 | 1 | 724 | 27,042 | | | | Proposed Conditions - Area 1 |
| 3 | Reservoir | 0.000 | 1 | 720 | 0 | 2 | 37.54 | 8,739 | Stormwater Basin 1 |
| 4 | SCS Runoff | 2.339 | 2 | 726 | 8,983 | | ***** | ***** | Existing Conditions - Area 2 |
| 5 | SCS Runoff | 6.085 | 2 | 724 | 19,344 | | | | Proposed Conditions - Area 2 |
| 6 | Reservoir | 0.000 | 2 | 840 | 0 | 5 | 37.73 | 6,653 | Stormwater Basin 2 |
| | | | | | | | | | |
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| A& | B Excavation | n TR55.g | pw | | Return I | eriod: 10 | Year | Thursday, | Apr 27, 2023 |

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Thursday, Apr 27, 2023

Hyd. No. 1

Existing Conditions - Areas 1

= SCS Runoff Hydrograph type Storm frequency = 10 yrsTime interval = 1 min Drainage area = 1.900 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 5.02 inStorm duration = 24 hrs

Peak discharge = 1.847 cfs
Time to peak = 726 min
Hyd. volume = 7,041 cuft
Curve number = 55*
Hydraulic length = 0 ft

Hydraulic length = 0 ft
Time of conc. (Tc) = 5.31 min
Distribution = Type III
Shape factor = 484

^{*} Composite (Area/CN) = [(4.100 x 55)] / 1.900



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

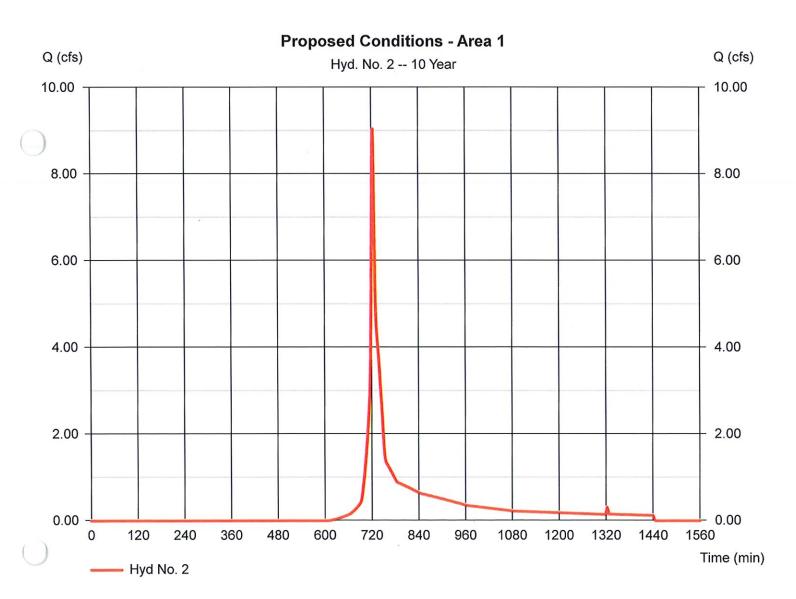
Thursday, Apr 27, 2023

Hyd. No. 2

Proposed Conditions - Area 1

= SCS Runoff = 9.052 cfsHydrograph type Peak discharge Storm frequency Time to peak = 10 yrs= 724 min Time interval Hyd. volume = 1 min = 27,042 cuftCurve number Drainage area = 4.100 ac= 67* Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 4.50 min= TR55 Total precip. = 5.02 inDistribution = Type III Shape factor = 484 Storm duration = 24 hrs

^{*} Composite (Area/CN) = [(1.800 x 83) + (2.300 x 55)] / 4.100



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Thursday, Apr 27, 2023

Hyd. No. 3

Stormwater Basin 1

Hydrograph type = Reservoir Storm frequency = 10 yrs Time interval = 1 min

Inflow hyd. No. = 2 - Proposed Conditions - Area 1

Reservoir name = Stormwater Basin 1

Peak discharge

= 0.000 cfs

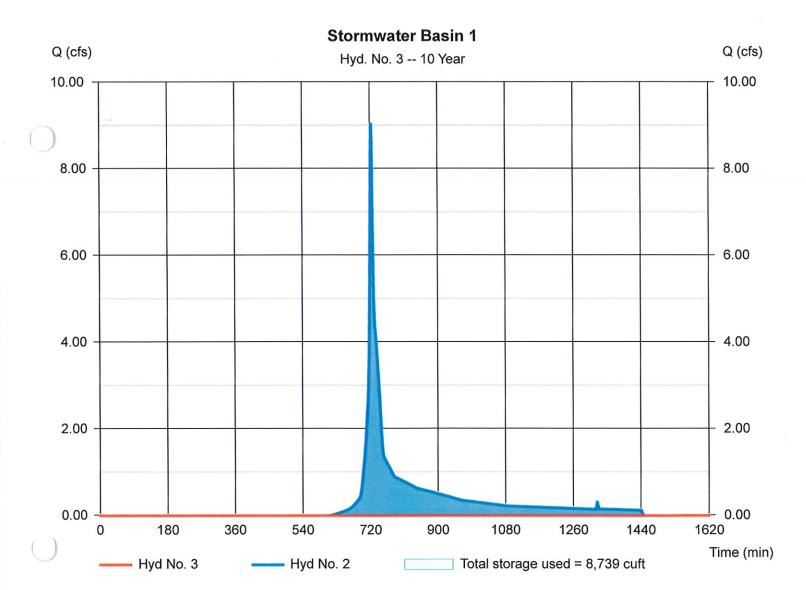
Time to peak Hyd. volume

= 720 min = 0 cuft

Max. Elevation

= 37.54 ft

Max. Storage = 8,739 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Thursday, Apr 27, 2023

Hyd. No. 4

Existing Conditions - Area 2

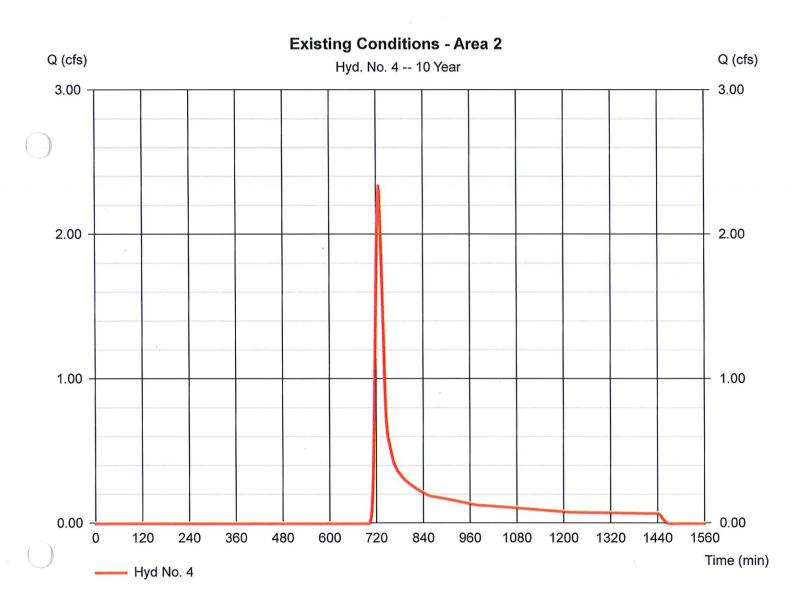
Hydrograph type = SCS Runoff Storm frequency = 10 yrsTime interval = 2 min Drainage area = 2.500 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 5.02 inStorm duration = 24 hrs

Peak discharge = 2.339 cfs Time to peak = 726 min Hyd. volume = 8,983 cuft Curve number = 55*

Curve number = 55* Hydraulic length = 0 ft

Time of conc. (Tc) = 16.90 min
Distribution = Type II
Shape factor = 484

^{*} Composite (Area/CN) = [(2.500 x 55)] / 2.500



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Thursday, Apr 27, 2023

Hyd. No. 5

Storm duration

Proposed Conditions - Area 2

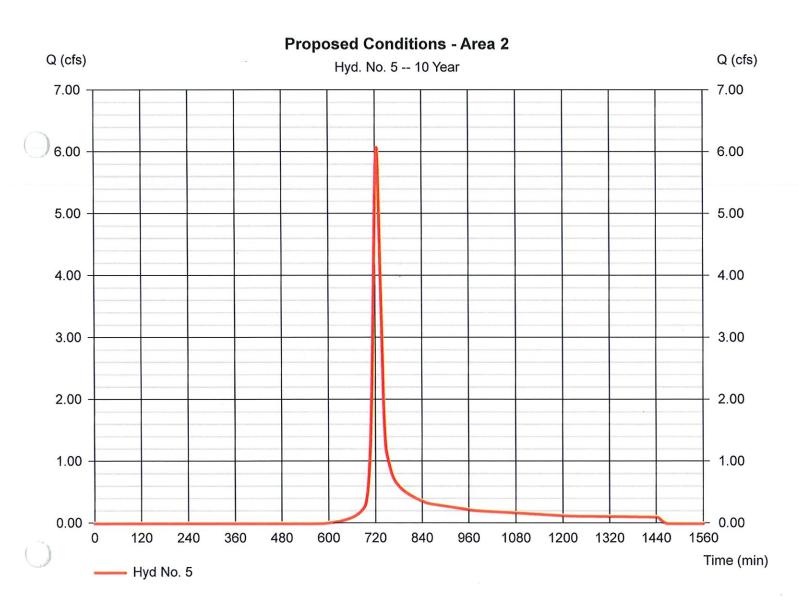
Hydrograph type = SCS Runoff
Storm frequency = 10 yrs
Time interval = 2 min
Drainage area = 2.500 ac
Basin Slope = 0.0 %
Tc method = TR55
Total precip. = 5.02 in

Peak discharge = 6.085 cfs Time to peak = 724 min Hyd. volume = 19,344 cuft

Curve number = 71* Hydraulic length = 0 ft

Time of conc. (Tc) = 16.49 min
Distribution = Type II
Shape factor = 484

= 24 hrs



^{*} Composite (Area/CN) = [(1.400 x 83) + (1.100 x 55)] / 2.500

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Thursday, Apr 27, 2023

Hyd. No. 6

Stormwater Basin 2

Hydrograph type = Reservoir Storm frequency = 10 yrs Time interval = 2 min

Inflow hyd. No. = 5 - Proposed Conditions - Area 2

Reservoir name = Stormwater Basin 2

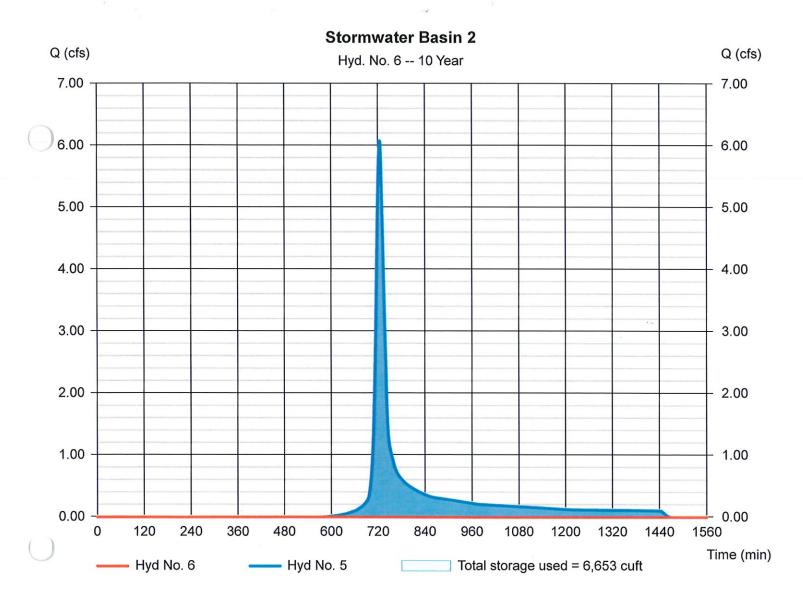
Peak discharge

= 0.000 cfs

Time to peak Hyd. volume = 840 min = 0 cuft

Max. Elevation
Max. Storage

= 37.73 ft = 6,653 cuft



Hydrograph Summary Report
Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

| /d. | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to peak (min) | Hyd. volume (cuft) | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph description |
|------------|--------------------------------|-----------------------|---------------------------|--------------------------|--------------------------|------------------|--|-------------------------------|-------------------------------|
| 1 | SCS Runoff | 3.208 | 1 | 725 | 11,041 | | | | Existing Conditions - Areas 1 |
| 2 | SCS Runoff | 13.05 | 1 | 724 | 38,330 | | | | Proposed Conditions - Area 1 |
| 3 | Reservoir | 0.000 | 1 | 977 | 0 | 2 | 38.30 | 13,877 | Stormwater Basin 1 |
| 4 | SCS Runoff | 4.056 | 2 | 726 | 14,088 | | | ****** | Existing Conditions - Area 2 |
| 5 | SCS Runoff | 8.514 | 2 | 724 | 26,746 | | ****** | | Proposed Conditions - Area 2 |
| 6 | Reservoir | 0.000 | 2 | 894 | 0 | 5 | 38.43 | 10,065 | Stormwater Basin 2 |
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| A& | B Excavation | n TR55.g | lpw | | Return I | Period: 25 | Year | Thursday, | Apr 27, 2023 |

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Thursday, Apr 27, 2023

Hyd. No. 1

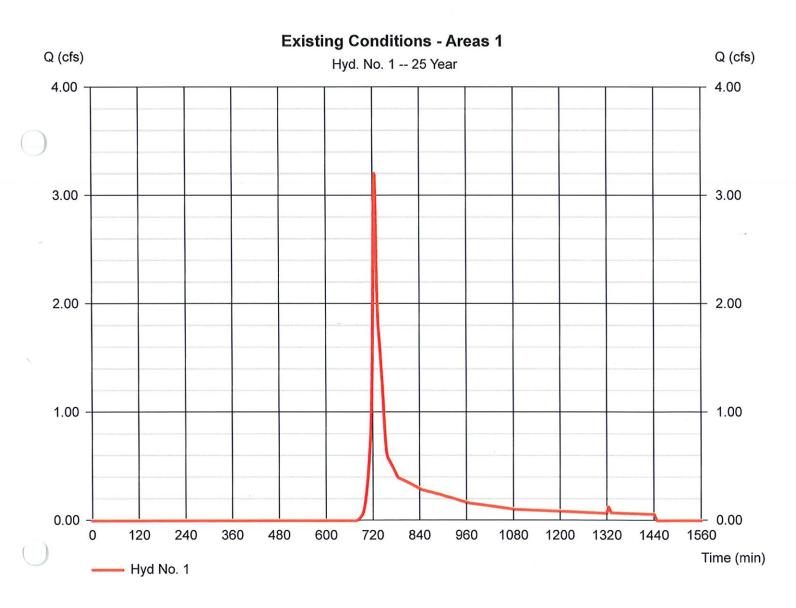
Existing Conditions - Areas 1

= SCS Runoff Hydrograph type Storm frequency = 25 yrsTime interval = 1 min Drainage area = 1.900 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 6.06 inStorm duration = 24 hrs

Peak discharge = 3.208 cfs
Time to peak = 725 min
Hyd. volume = 11,041 cuft
Curve number = 55*
Hydraulic length = 0 ft
Time of conc. (Tc) = 5.31 min

Distribution = Type III Shape factor = 484

^{*} Composite (Area/CN) = [(4.100 x 55)] / 1.900



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Thursday, Apr 27, 2023

Hyd. No. 2

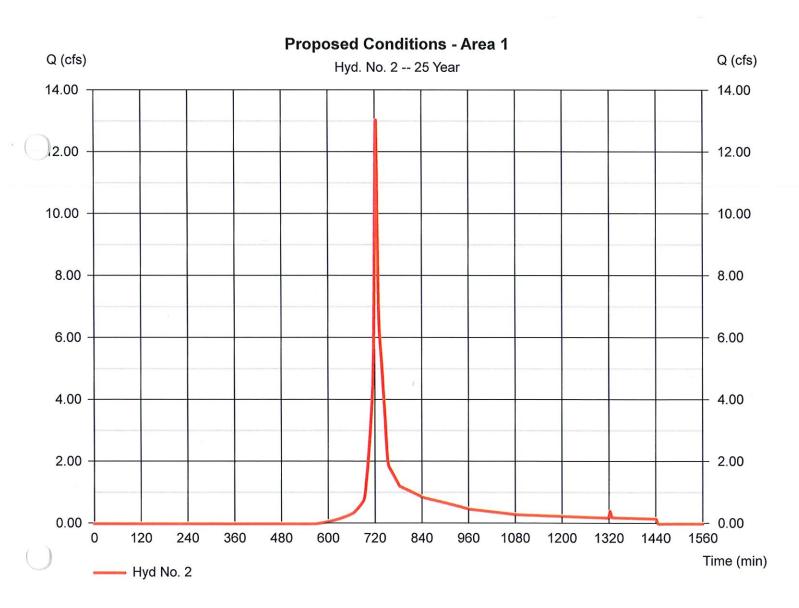
Proposed Conditions - Area 1

Hydrograph type = SCS Runoff Storm frequency = 25 yrsTime interval = 1 min Drainage area = 4.100 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 6.06 inStorm duration = 24 hrs

Peak discharge = 13.05 cfs Time to peak = 724 min Hyd. volume = 38,330 cuft

Curve number = 67*
Hydraulic length = 0 ft
Time of conc. (Tc) = 4.50 min
Distribution = Type III
Shape factor = 484

^{*} Composite (Area/CN) = [(1.800 x 83) + (2.300 x 55)] / 4.100



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= 0.000 cfs

= 977 min

= 0 cuft

Hyd. No. 3

Stormwater Basin 1

Hydrograph type = Reservoir Storm frequency = 25 yrs Time interval = 1 min

Inflow hyd. No. Reservoir name = Stormwater Basin 1

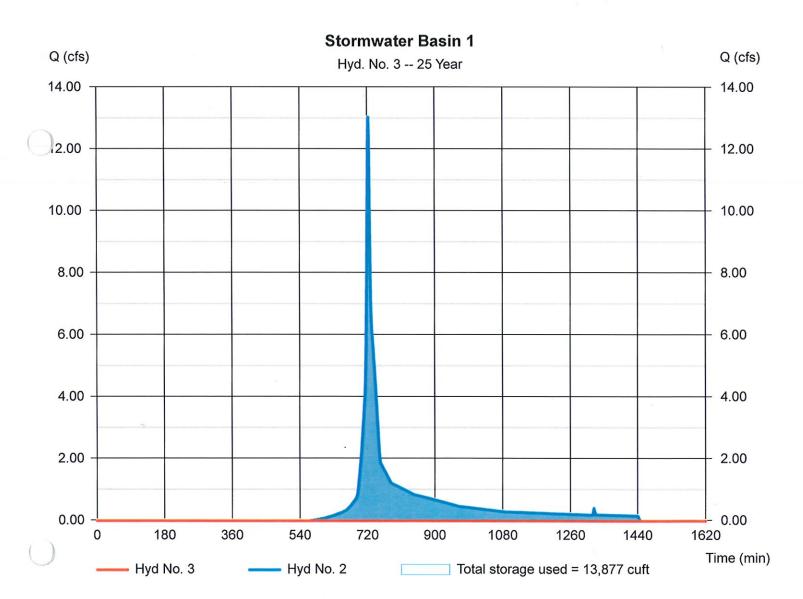
= 2 - Proposed Conditions - Area 1

Max. Elevation = 38.30 ftMax. Storage = 13,877 cuft

Peak discharge

Time to peak

Hyd. volume



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Hyd. No. 4

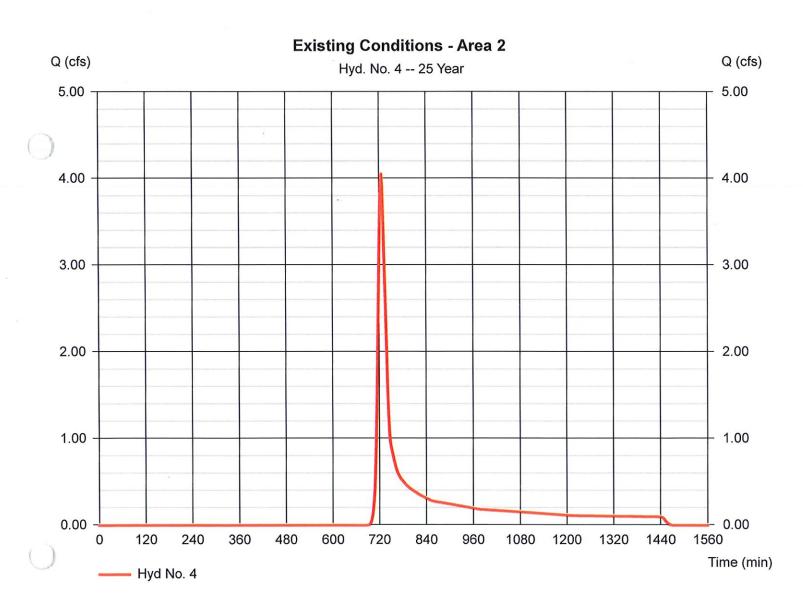
Existing Conditions - Area 2

Hydrograph type = SCS Runoff = 25 yrs Storm frequency Time interval = 2 min Drainage area = 2.500 acBasin Slope = 0.0 % Tc method = TR55 Total precip. = 6.06 inStorm duration = 24 hrs

Peak discharge = 4.056 cfs
Time to peak = 726 min
Hyd. volume = 14,088 cuft
Curve number = 55*

Hydraulic length = 0 ft
Time of conc. (Tc) = 16.90 min
Distribution = Type II
Shape factor = 484

^{*} Composite (Area/CN) = [(2.500 x 55)] / 2.500



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Thursday, Apr 27, 2023

Hyd. No. 5

Proposed Conditions - Area 2

Hydrograph type = SCS Runoff = 25 yrs Storm frequency Time interval = 2 min Drainage area = 2.500 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 6.06 inStorm duration = 24 hrs

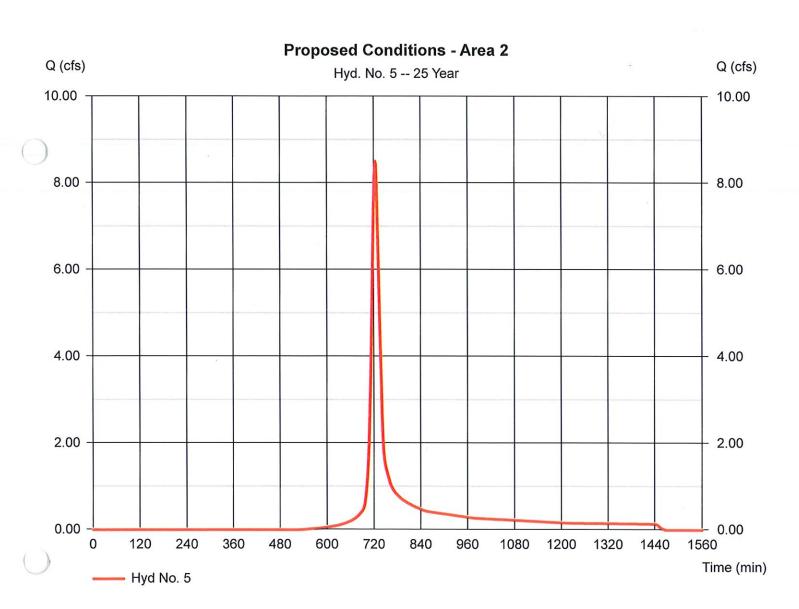
= 8.514 cfsPeak discharge Time to peak = 724 min Hyd. volume = 26,746 cuft

Curve number = 71* Hydraulic length = 0 ft

Distribution

Time of conc. (Tc) = 16.49 min= Type II Shape factor = 484

^{*} Composite (Area/CN) = [(1.400 x 83) + (1.100 x 55)] / 2.500



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Thursday, Apr 27, 2023

Hyd. No. 6

Stormwater Basin 2

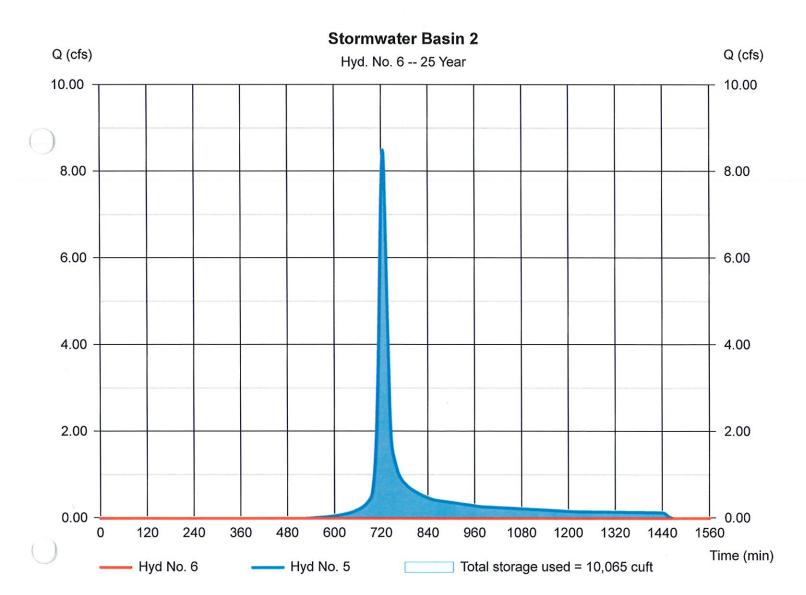
Hydrograph type = Reservoir Storm frequency = 25 yrs Time interval = 2 min

Inflow hyd. No. = 5 - Proposed Conditions - Area 2

Reservoir name = Stormwater Basin 2

Peak discharge = 0.000 cfs Time to peak = 894 min

Hyd. volume = 0 cuft
Max. Elevation = 38.43 ft
Max. Storage = 10,065 cuft



Hydrograph Summary Report
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

| ˈyd. ۥlo. | Hydrograph type (origin) | Peak flow (cfs) | Time interval (min) | Time to peak (min) | Hyd. volume (cuft) | Inflow hyd(s) | Maximum elevation (ft) | Total strge used (cuft) | Hydrograph description |
|--------------|--------------------------------|-----------------------|---------------------------|--------------------------|--------------------------|--|------------------------------|-------------------------------|-------------------------------|
| 1 | SCS Runoff | 5.659 | 1 | 725 | 18,215 | | ****** | | Existing Conditions - Areas 1 |
| 2 | SCS Runoff | 19.75 | 1 | 723 | 57,285 | ****** | **** | | Proposed Conditions - Area 1 |
| 3 | Reservoir | 5.325 | 1 | 738 | 7,823 | 2 | 38.85 | 17,846 | Stormwater Basin 1 |
| 4 | SCS Runoff | 7.095 | 2 | 726 | 23,240 | | | | Existing Conditions - Area 2 |
| 5 | SCS Runoff | 12.46 | 2 | 724 | 38,967 | | | P | Proposed Conditions - Area 2 |
| 6 | Reservoir | 6.326 | 2 | 734 | 5,602 | 5 | 38.83 | 12,131 | Stormwater Basin 2 |
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| A Company | | | | | | | | | |
| A& | A&B Excavation TR55.gpw | | | | | Return Period: 100 Year Thursday, Apr 27, 2023 | | | |

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Thursday, Apr 27, 2023

Hyd. No. 1

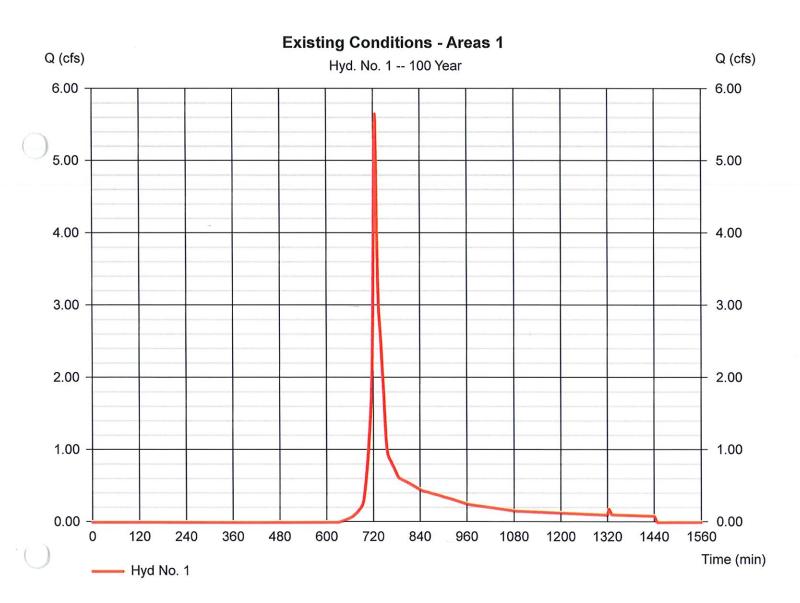
Existing Conditions - Areas 1

= SCS Runoff Hydrograph type Storm frequency = 100 yrsTime interval = 1 min Drainage area = 1.900 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 7.67 inStorm duration = 24 hrs

Peak discharge = 5.659 cfs
Time to peak = 725 min
Hyd. volume = 18,215 cuft
Curve number = 55*

Hydraulic length = 0 ft
Time of conc. (Tc) = 5.31 min
Distribution = Type III
Shape factor = 484

^{*} Composite (Area/CN) = [(4.100 x 55)] / 1.900



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Hyd. No. 2

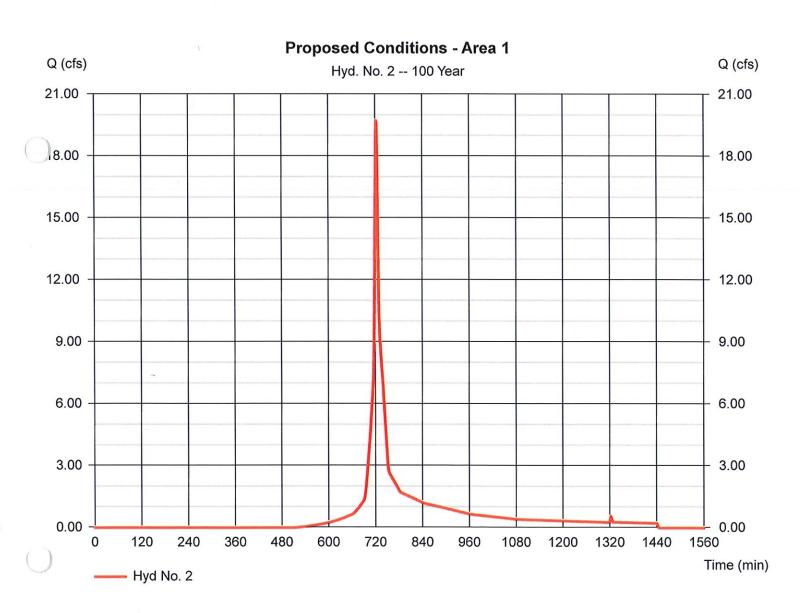
Proposed Conditions - Area 1

Hydrograph type = SCS Runoff = 100 yrs Storm frequency Time interval = 1 min Drainage area = 4.100 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 7.67 inStorm duration = 24 hrs

Peak discharge = 19.75 cfs
Time to peak = 723 min
Hyd. volume = 57,285 cuft
Curve number = 67*

Hydraulic length = 0 ft
Time of conc. (Tc) = 4.50 min
Distribution = Type III
Shape factor = 484

^{*} Composite (Area/CN) = [(1.800 x 83) + (2.300 x 55)] / 4.100



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= 5.325 cfs

= 7,823 cuft

= 738 min

Hyd. No. 3

Stormwater Basin 1

Hydrograph type = Reservoir Storm frequency = 100 yrsTime interval = 1 min

Inflow hyd. No. Reservoir name = Stormwater Basin 1

= 2 - Proposed Conditions - Area 1

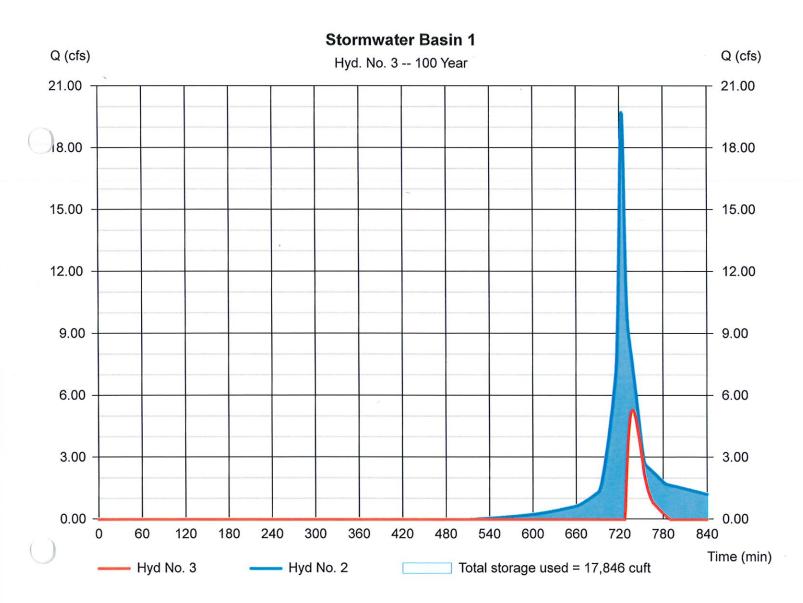
Max. Storage

Peak discharge

Time to peak

Hyd. volume

Max. Elevation = 38.85 ft= 17,846 cuft



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Hyd. No. 4

Existing Conditions - Area 2

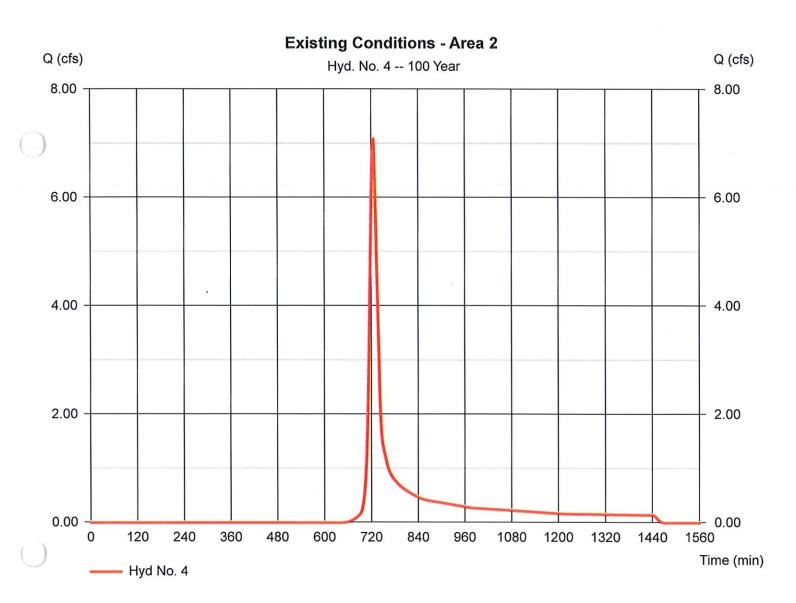
Hydrograph type = SCS Runoff = 100 yrs Storm frequency Time interval = 2 min Drainage area = 2.500 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 7.67 inStorm duration = 24 hrs

Peak discharge = 7.095 cfs
Time to peak = 726 min
Hyd. volume = 23,240 cuft

Curve number = 55* Hydraulic length = 0 ft

Time of conc. (Tc) = 16.90 min
Distribution = Type II
Shape factor = 484

^{*} Composite (Area/CN) = [(2.500 x 55)] / 2.500



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Hyd. No. 5

Proposed Conditions - Area 2

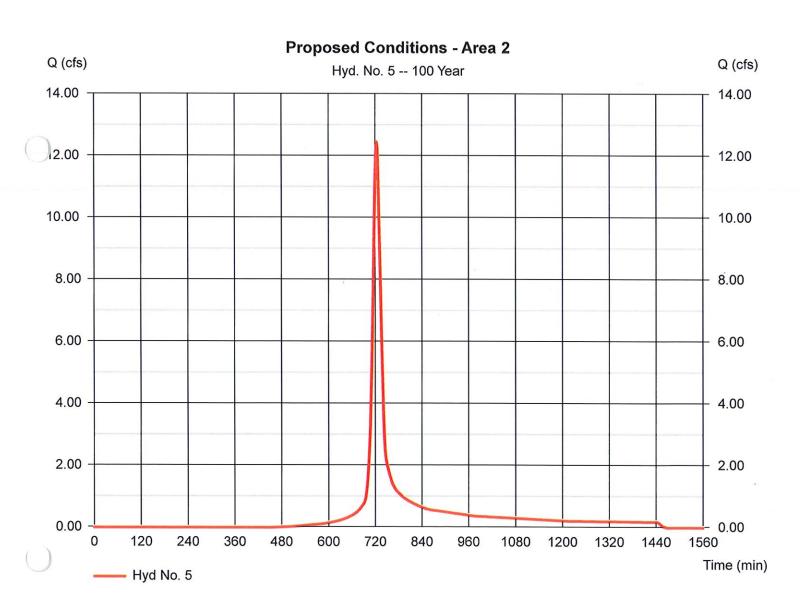
Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval = 2 min Drainage area = 2.500 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 7.67 inStorm duration = 24 hrs

Peak discharge = 12.46 cfs
Time to peak = 724 min
Hyd. volume = 38,967 cuft

Curve number = 71* Hydraulic length = 0 ft Time of conc. (Tc) = 16.49 min

Distribution = Type II
Shape factor = 484

^{*} Composite (Area/CN) = [(1.400 x 83) + (1.100 x 55)] / 2.500



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Hyd. No. 6

Stormwater Basin 2

Hydrograph type = Reservoir Storm frequency = 100 yrs Time interval = 2 min

Inflow hyd. No. = 5 - Proposed Conditions - Area 2

Reservoir name = Stormwater Basin 2

Peak discharge = 6.326 cfs Time to peak = 734 min

Hyd. volume = 5,602 cuft
Max. Elevation = 38.83 ft

Max. Storage = 12,131 cuft

