# DRAINAGE CALCULATIONS, HYDRAULICS & HYDROLOGY REPORT

1492 Hartford – New London Turnpike (CT Route 85) Montville, CT

April 8<sup>th</sup>, 2024

#### DRAINAGE HYDRAULICS AND HYDROLOGY REPORT

## 1492 Hartford – New London Turnpike (CT Route 85) Montville, CT

#### **EXISTING CONDITIONS**

The site is approximately 5.62 acres in area and is shown on the Existing Conditions Survey (Sheet 1 of the site plans). The site has frontage on Hartford – New London Turnpike (Route 85). There are approximately 0.38 acres of wetlands on the site.

#### PROPOSED DEVELOPMENT

The project proposes the development of a processing, material storage, and equipment storage facility. There will be no free standing buildings on the site but there will be several storage bays and a construction trailer.

The 5.62 acres site contains wetlands as shown on sheet 1. Of the 5.62 acres, 4.08 acres will be disturbed during the development process. There will be no disturbance within the wetlands or upland review area.

#### **EXISTING AND PROPOSED HYDRAULICS**

The stormwater management system has been designed to provide for zero increase in peak stormwater discharge from the site. The project has been designed to actually result in a decrease in the peak stormwater rates leaving the project site. The proposed stormwater water quality basin will provide treatment of the runoff from the proposed site.

The current site is divided into two, existing, drainage areas:

Existing Drainage Area 1	2.98 Acres
Existing Drainage Area 2	2.64 Acres

The development of the proposed site will result in two drainage areas:

Proposed Drainage Area 1	4.08 Acres			
Proposed Drainage Area 2	1.54 Acres			

Proposed Drainage Area 1 contains the developed site. The stormwater runoff from this area will be treated by the water quality basin in the northwestern corner of the site. Proposed Drainage Area 2 contains the wetlands and upland review area and will remain undeveloped.

Both the existing and the proposed conditions for the development site have been analyzed for the 2-year, 10-year, 25-year, 50-year, and 100 year design storms using the Rational Method.

#### **Drainage Area 1**

	2 Year	10 Year	25 Year	50 Year	100 Year
Existing	2.21 cfs	3.32 cfs	4.00 cfs	4.50 cfs	5.04 cfs
Proposed	0.00 cfs				

#### Drainage Area 2

	2 Year	10 Year	25 Year	50 Year	100 Year
Existing	1.96 cfs	2.94 cfs	3.54 cfs	3.99 cfs	4.46 cfs
Proposed	0.70 cfs	1.05 cfs	1.27 cfs	1.43 cfs	1.60 cfs

**EROSION & SEDIMENTATION CONTROL** 

The 2002 CT Guidelines for Soil Erosion & Sedimentation Control applies to the

construction phase of the project. A detailed erosion and sediment control plan has been

provided in the site development plans. The proposed stormwater water quality basin has

been designed to function as sedimentation traps during stabilization.

The first calculation required by the Guidelines is for the sediment storage volume

(SSV). The sediment storage volume is the calculation for one year of predicted sediment

load. The required SSV calculation for the temporary sediment trap is shown below.

Drainage Area 1A

SSV = A(134CY/Acre)

A = 4.08 ACRE

SSV = 546.72 CY = 14,760 CF

The second calculation required by the Guidelines is for wet storage volume

(WSV). The wet storage volume is the volume in the basin that is located below the

bottom of the riprap for the level spreader outlet of the basin. The volume of the wet

storage is required to be half of the required SSV. The required wet storage volume is

shown below along with the dry storage volumes (DSV).

Drainage Area 1A

WSV = DSV = SSV/2

= 7,380 CF

H-3

The required and provided storage for each basin are as follows:

**Drainage Area 1** (Bottom of Riprap Elev. = 206.5)

Sedimentation Trap

7,380 CF of Wet Storage Volume Required 11,965 CF Provided

7,380 CF of Dry Storage Volume Required 10,260 CF Provided

14,760 CF of Sediment Storage Volume Required 22,225 CF Total Provided

#### CONNECTICUT STORMWATER QUALITY MANUAL

The Connecticut 2004 Stormwater Quality Manual (Manual) applies to the post construction phase, for the operation of the facility. The temporary sediment traps have been designed to function as water quality basins after the site is stabilized. They all meet the criteria of the Connecticut Stormwater Quality Manual for a Water Quality Basin.

#### **Drainage Area 1**

WQV = (1")(R)(A)/12

A = 4.08 Acre

R = 0.05 + 0.009(I)

I = 0.05 Acres / 4.08 Acres = 0.012 (1.2%)

R = 0.061

WQV = 0.021 Ac-Ft = 903.9 CF (Required)

11,965 CF (Provided)

Once development of the site is completed, there will be a decrease in runoff from the site. The temporary sedimentation basin provides ample wet and dry storage volume to meet and exceed the requirements of the 2002 CT Guidelines for Soil & Sedimentation Control. Likewise, Water Quality Basin meets and exceeds the post construction requirements of the Connecticut 2004 Stormwater Quality Manual.

# Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

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Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	Rational	2.212	1	9	1,194				Existing Area 1
2	Rational	3.028	1	9	1,635				Proposed Area 1
3	Reservoir	0.000	1	n/a	0	2	204.43	1,635	Water Quality Basin
4	Rational	1.960	1	9	1,058				Existing Area 2
5	Rational	0.702	1	22	926				Proposed Area 2
GSI	GSD 69 - Drainage Calculations - V1.gpw					eriod: 2 Ye	ar	Friday, Apr	5, 2024

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Friday, Apr 5, 2024

= 2.212 cfs

= 1,194 cuft

 $= 9.00 \, \text{min}$ 

= 9 min

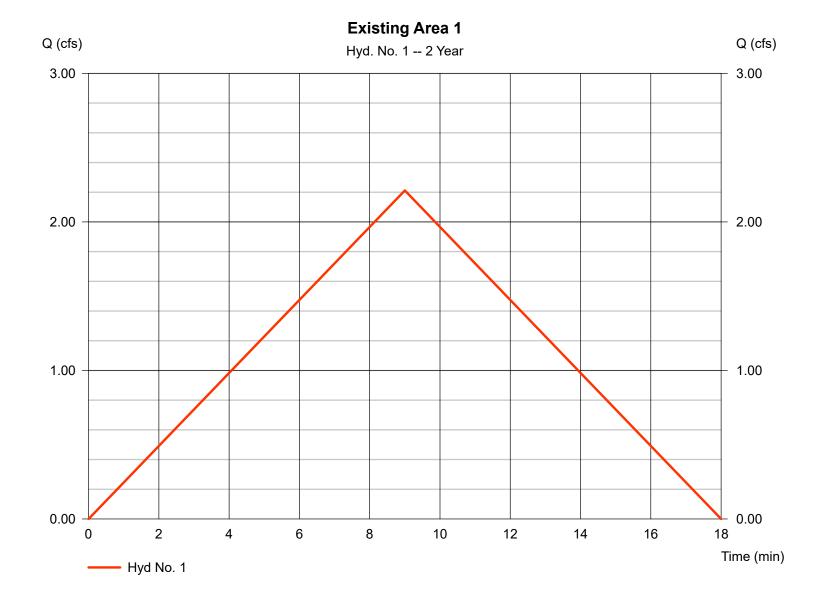
= 0.2

= 1/1

### Hyd. No. 1

**Existing Area 1** 

Hydrograph type Peak discharge = Rational Storm frequency Time to peak = 2 yrs = 1 min Time interval Hyd. volume Drainage area = 2.980 acRunoff coeff. Intensity = 3.711 in/hrTc by TR55 IDF Curve = GSD-60 NOAA.IDF Asc/Rec limb fact



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#### Hyd. No. 2

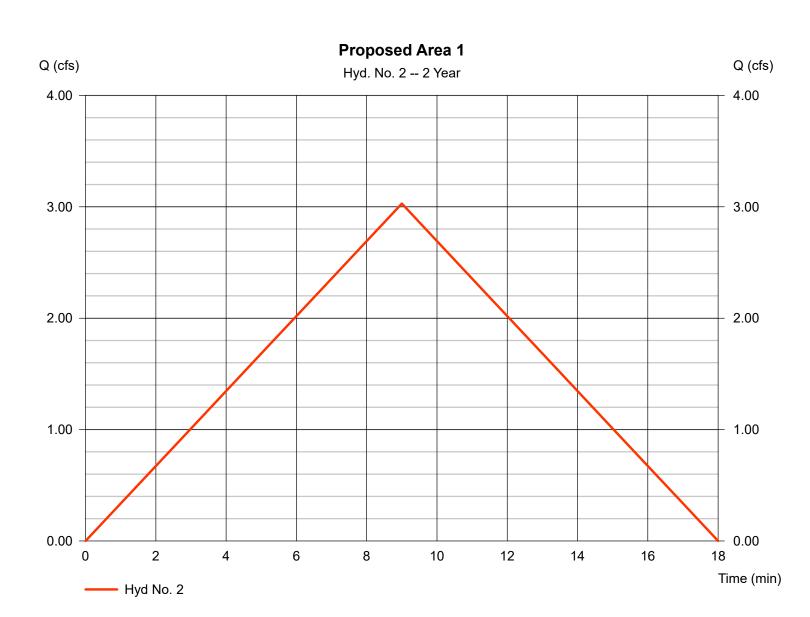
Proposed Area 1

Hydrograph type = Rational
Storm frequency = 2 yrs
Time interval = 1 min
Drainage area = 4.080 ac
Intensity = 3.711 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 3.028 cfs
Time to peak = 9 min
Hyd. volume = 1,635 cuft
Runoff coeff. = 0.2

Tc by TR55 = 9.00 min Asc/Rec limb fact = 1/1



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### Hyd. No. 3

Water Quality Basin

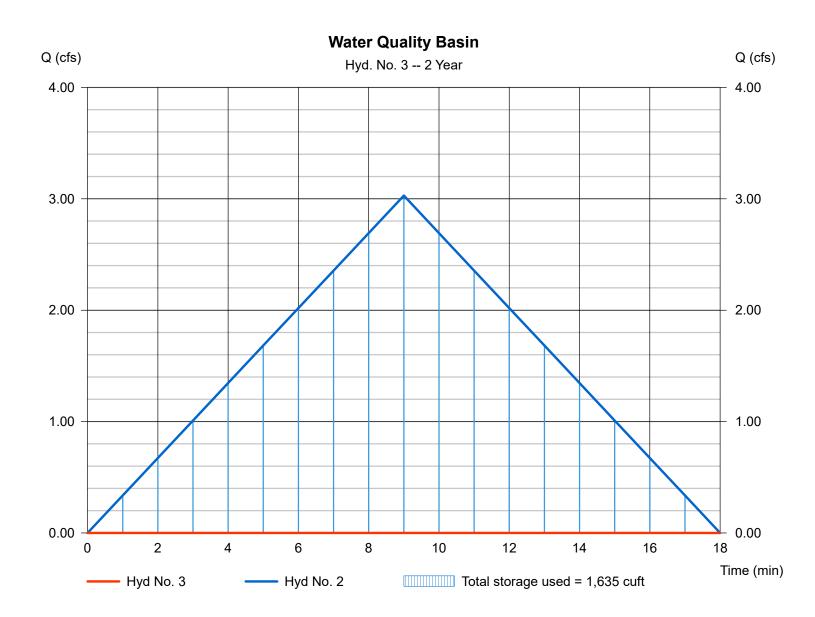
Hydrograph type = Reservoir Storm frequency = 2 yrs Time interval = 1 min

Inflow hyd. No. = 2 - Proposed Area 1

Reservoir name = Pond 1

Peak discharge = 0.000 cfs
Time to peak = n/a
Hyd. volume = 0 cuft
Max. Elevation = 204.43 ft
Max. Storage = 1,635 cuft

Storage Indication method used.



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## Hyd. No. 4

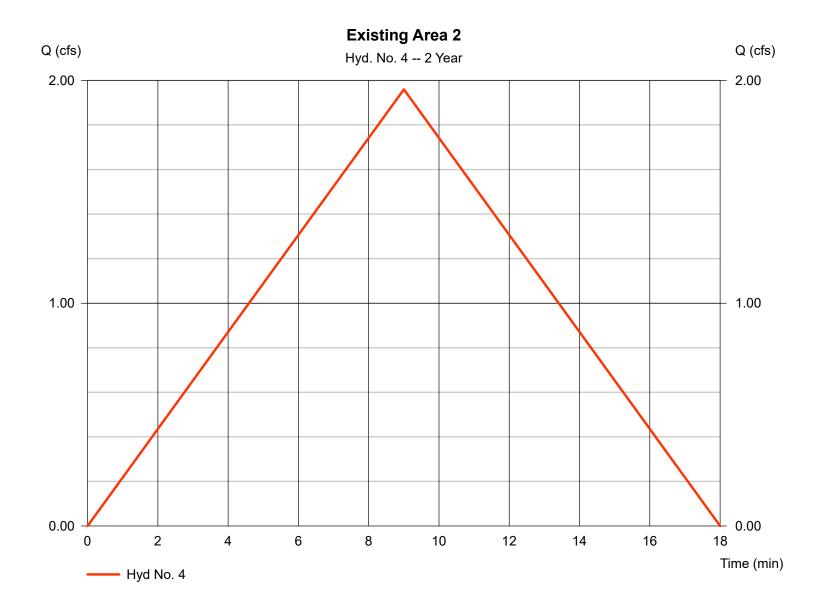
**Existing Area 2** 

Hydrograph type = Rational
Storm frequency = 2 yrs
Time interval = 1 min
Drainage area = 2.640 ac
Intensity = 3.711 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 1.960 cfs
Time to peak = 9 min
Hyd. volume = 1,058 cuft

Runoff coeff. = 0.2 Tc by TR55 = 9.00 min



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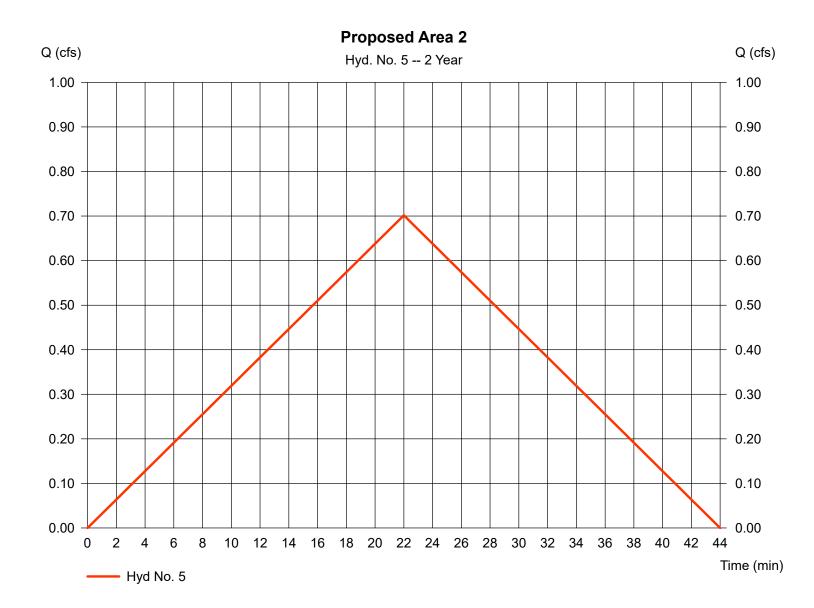
Friday, Apr 5, 2024

### Hyd. No. 5

Proposed Area 2

Hydrograph type = Rational Storm frequency = 2 yrs Time interval = 1 min Drainage area = 1.540 acIntensity = 2.278 in/hr

IDF Curve = GSD-60 NOAA.IDF Peak discharge = 0.702 cfsTime to peak = 22 min Hyd. volume = 926 cuft Runoff coeff. = 0.2Tc by TR55 = 22.00 min



# Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

		,		2000 27 (2000) 2000 27 (2000)					
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	Rational	2.807	1	9	1,516				Existing Area 1
2	Rational	3.842	1	9	2,075				Proposed Area 1
3	Reservoir	0.000	1	n/a	0	2	204.54	2,075	Water Quality Basin
4	Rational	2.486	1	9	1,343				Existing Area 2
5	Rational	0.891	1	22	1,176				Proposed Area 2
GSI	GSD 69 - Drainage Calculations - V1.gpw					eriod: 5 Ye	ar	Friday, Apr	5, 2024

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#### Hyd. No. 1

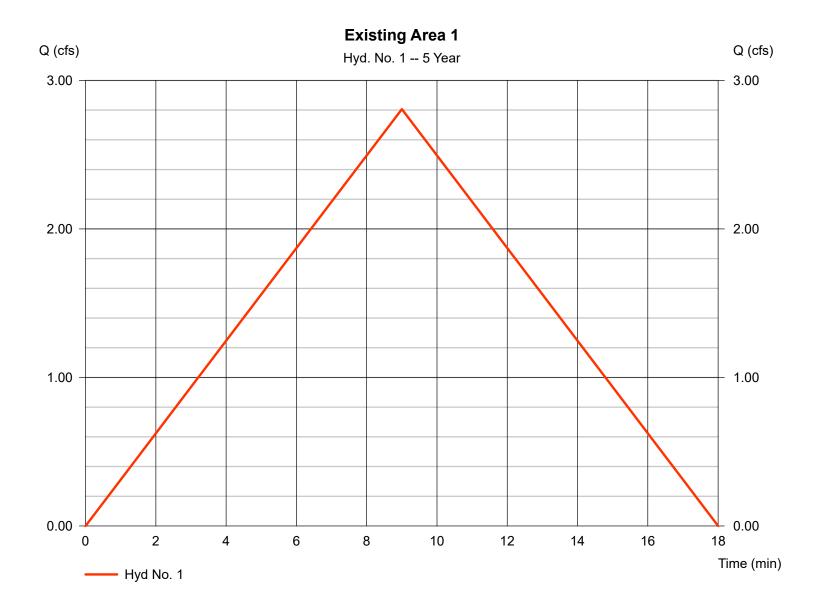
**Existing Area 1** 

Hydrograph type = Rational
Storm frequency = 5 yrs
Time interval = 1 min
Drainage area = 2.980 ac
Intensity = 4.709 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 2.807 cfs
Time to peak = 9 min
Hyd. volume = 1,516 cuft

Runoff coeff. = 0.2 Tc by TR55 = 9.00 min



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#### Hyd. No. 2

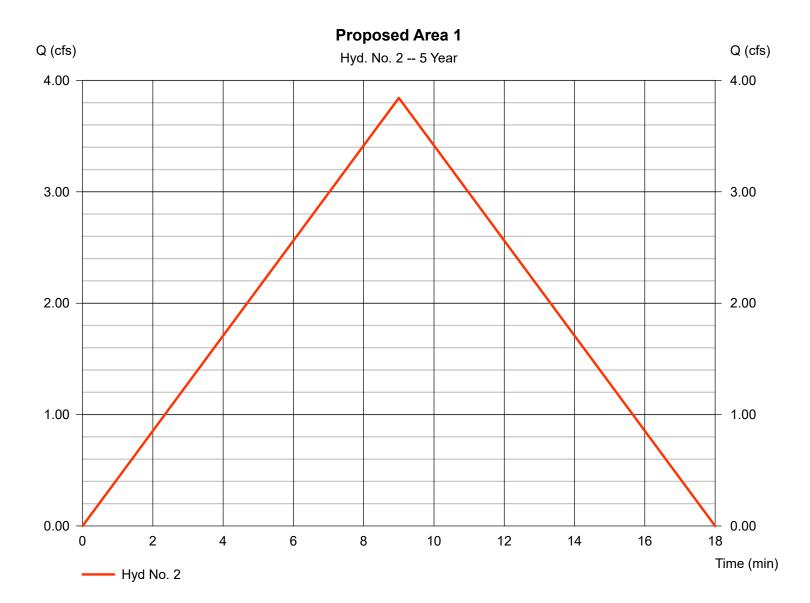
Proposed Area 1

Hydrograph type = Rational
Storm frequency = 5 yrs
Time interval = 1 min
Drainage area = 4.080 ac
Intensity = 4.709 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 3.842 cfs
Time to peak = 9 min
Hyd. volume = 2,075 cuft
Runoff coeff. = 0.2

Tc by TR55 = 9.00 min Asc/Rec limb fact = 1/1



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#### Hyd. No. 3

Water Quality Basin

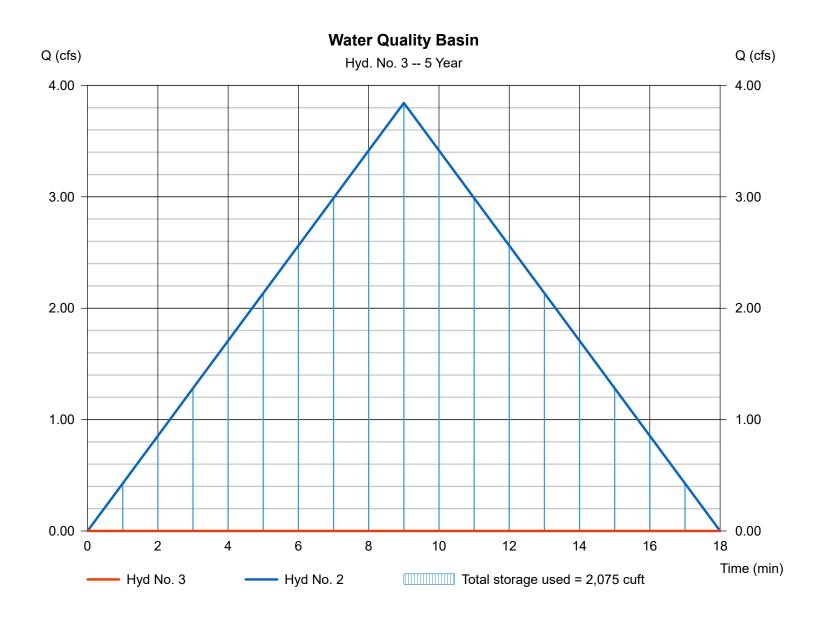
Hydrograph type = Reservoir Storm frequency = 5 yrs Time interval = 1 min

Inflow hyd. No. = 2 - Proposed Area 1

Reservoir name = Pond 1

Peak discharge = 0.000 cfs
Time to peak = n/a
Hyd. volume = 0 cuft
Max. Elevation = 204.54 ft
Max. Storage = 2,075 cuft

Storage Indication method used.



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## Hyd. No. 4

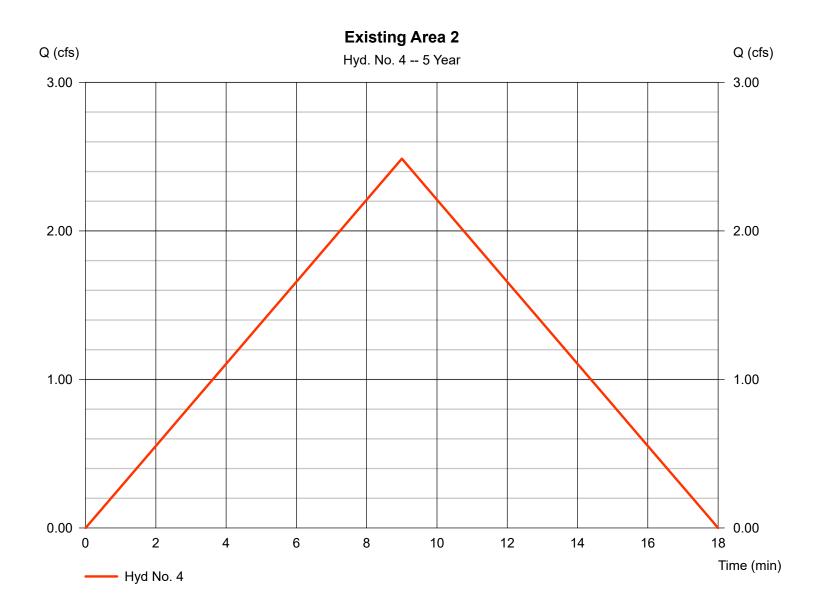
**Existing Area 2** 

Hydrograph type = Rational
Storm frequency = 5 yrs
Time interval = 1 min
Drainage area = 2.640 ac
Intensity = 4.709 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 2.486 cfs
Time to peak = 9 min
Hyd. volume = 1,343 cuft
Runoff coeff. = 0.2

Tc by TR55 = 9.00 min



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#### Hyd. No. 5

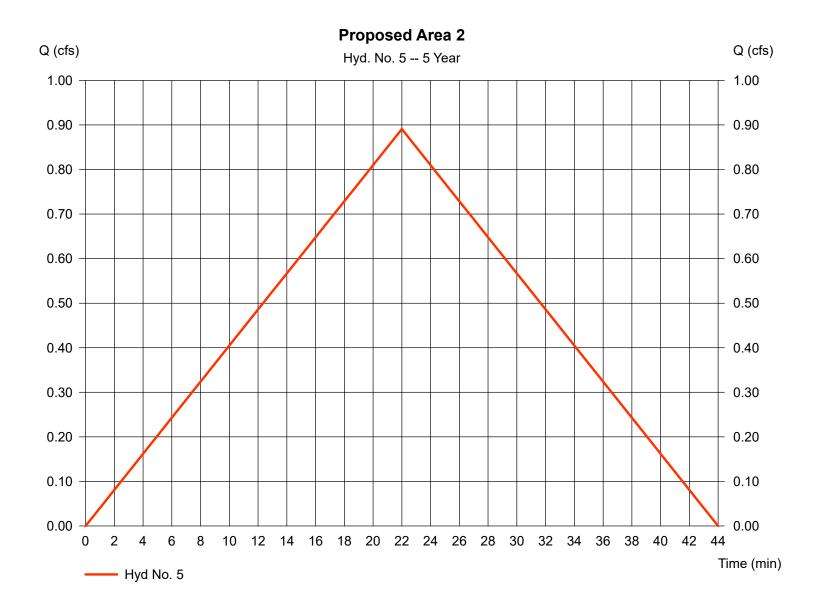
Proposed Area 2

Hydrograph type = Rational Storm frequency = 5 yrsTime interval = 1 min Drainage area = 1.540 ac

Intensity = 2.892 in/hr

IDF Curve = GSD-60 NOAA.IDF Peak discharge = 0.891 cfsTime to peak = 22 min Hyd. volume = 1,176 cuftRunoff coeff. = 0.2

Tc by TR55 = 22.00 min



# Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

		,		2000 277 (2000)					
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	Rational	3.318	1	9	1,792				Existing Area 1
2	Rational	4.543	1	9	2,453				Proposed Area 1
3	Reservoir	0.000	1	n/a	0	2	204.63	2,453	Water Quality Basin
4	Rational	2.940	1	9	1,587				Existing Area 2
5	Rational	1.052	1	22	1,388				Proposed Area 2
GSI	GSD 69 - Drainage Calculations - V1.gpw F					eriod: 10 Y	ear	Friday, Apr	5, 2024

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## Hyd. No. 1

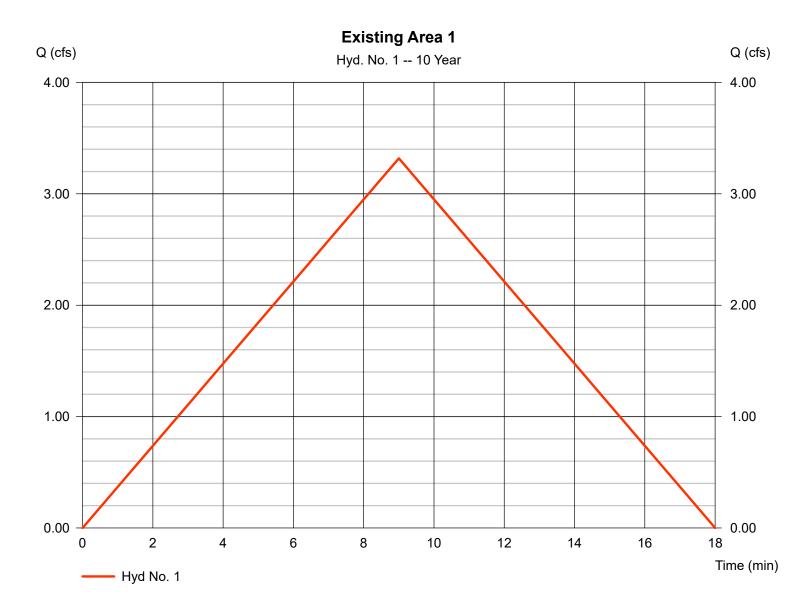
Existing Area 1

Hydrograph type = Rational
Storm frequency = 10 yrs
Time interval = 1 min
Drainage area = 2.980 ac
Intensity = 5.567 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 3.318 cfs
Time to peak = 9 min
Hyd. volume = 1,792 cuft
Runoff coeff. = 0.2

Tc by TR55 = 9.00 minAsc/Rec limb fact = 1/1



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#### Hyd. No. 2

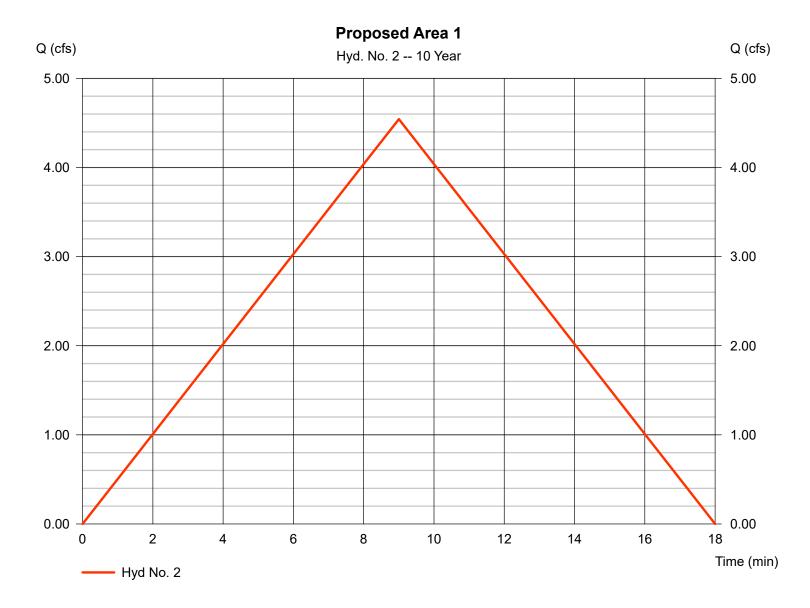
Proposed Area 1

Hydrograph type = Rational
Storm frequency = 10 yrs
Time interval = 1 min
Drainage area = 4.080 ac
Intensity = 5.567 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 4.543 cfs
Time to peak = 9 min
Hyd. volume = 2,453 cuft
Runoff coeff. = 0.2

Tc by TR55 = 9.00 min



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#### Hyd. No. 3

Water Quality Basin

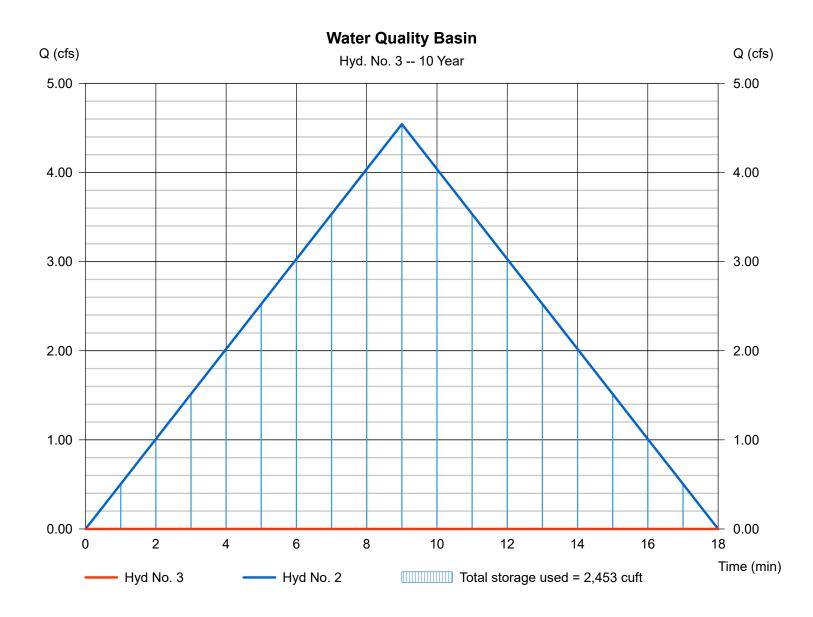
Hydrograph type = Reservoir Storm frequency = 10 yrs Time interval = 1 min

Inflow hyd. No. = 2 - Proposed Area 1

Reservoir name = Pond 1

Peak discharge = 0.000 cfs
Time to peak = n/a
Hyd. volume = 0 cuft
Max. Elevation = 204.63 ft
Max. Storage = 2,453 cuft

Storage Indication method used.



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## Hyd. No. 4

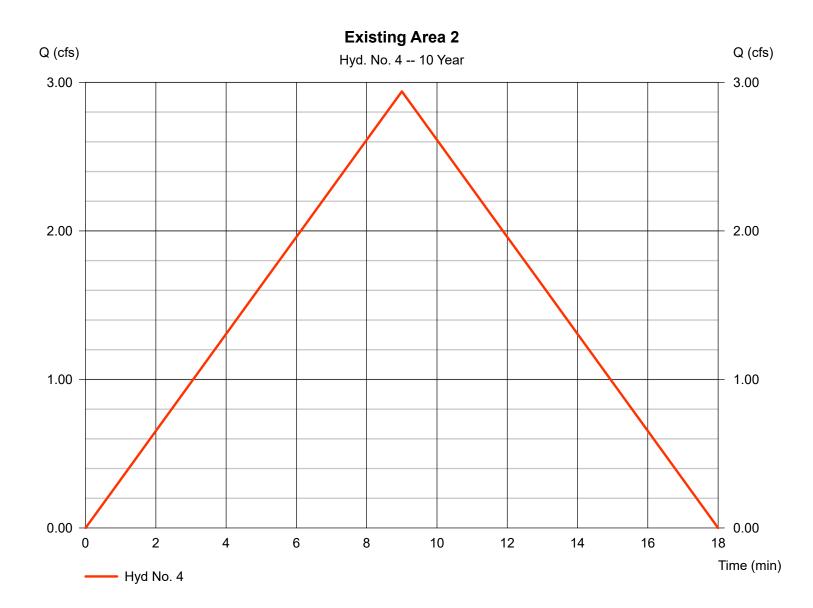
**Existing Area 2** 

Hydrograph type = Rational
Storm frequency = 10 yrs
Time interval = 1 min
Drainage area = 2.640 ac
Intensity = 5.567 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 2.940 cfs
Time to peak = 9 min
Hyd. volume = 1,587 cuft
Runoff coeff. = 0.2

Tc by TR55 = 9.00 min



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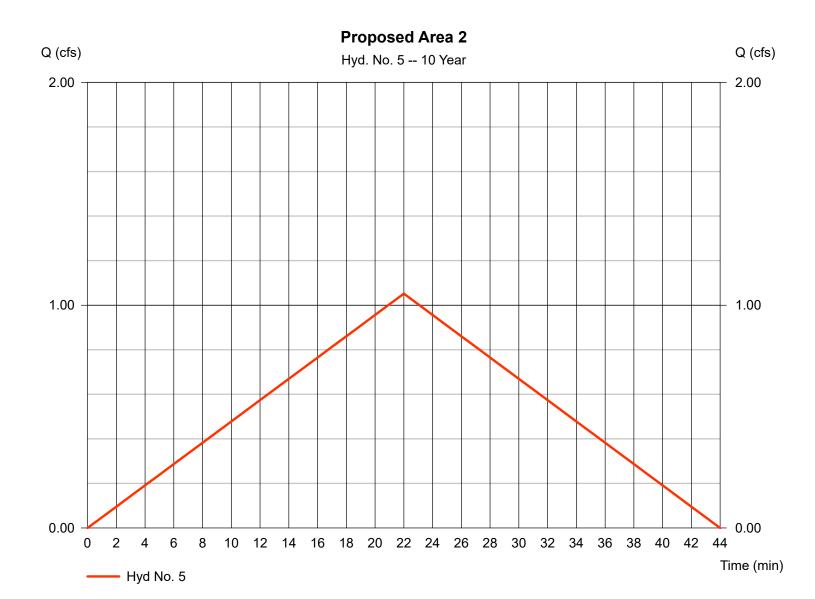
### Hyd. No. 5

Proposed Area 2

Hydrograph type Peak discharge = 1.052 cfs= Rational Storm frequency Time to peak = 10 yrs= 22 min Time interval = 1 min Hyd. volume = 1,388 cuft Drainage area = 1.540 acRunoff coeff. = 0.2

Intensity = 3.415 in/hr Tc by TR55 = 22.00 min

IDF Curve = GSD-60 NOAA.IDF Asc/Rec limb fact = 1/1



# Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

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Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	Rational	3.999	1	9	2,159				Existing Area 1
2	Rational	5.475	1	9	2,956				Proposed Area 1
3	Reservoir	0.000	1	n/a	0	2	204.74	2,956	Water Quality Basin
4	Rational	3.543	1	9	1,913				Existing Area 2
5	Rational	1.268	1	22	1,674				Proposed Area 2
GS	GSD 69 - Drainage Calculations - V1.gpw F					eriod: 25 Y	ear ear	Friday, Apr	5, 2024

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#### Hyd. No. 1

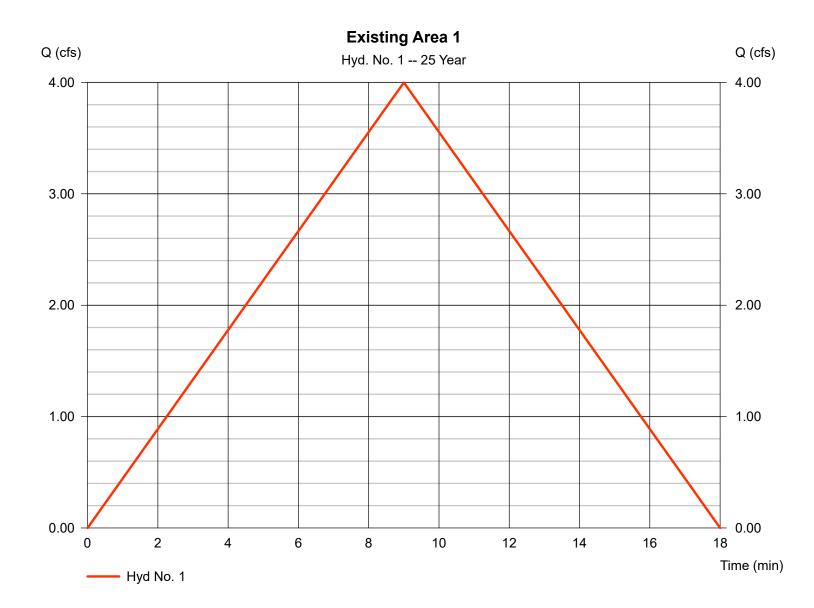
Existing Area 1

Hydrograph type = Rational
Storm frequency = 25 yrs
Time interval = 1 min
Drainage area = 2.980 ac
Intensity = 6.710 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 3.999 cfs
Time to peak = 9 min
Hyd. volume = 2,159 cuft
Runoff coeff. = 0.2

Tc by TR55 = 9.00 minAsc/Rec limb fact = 1/1



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#### Hyd. No. 2

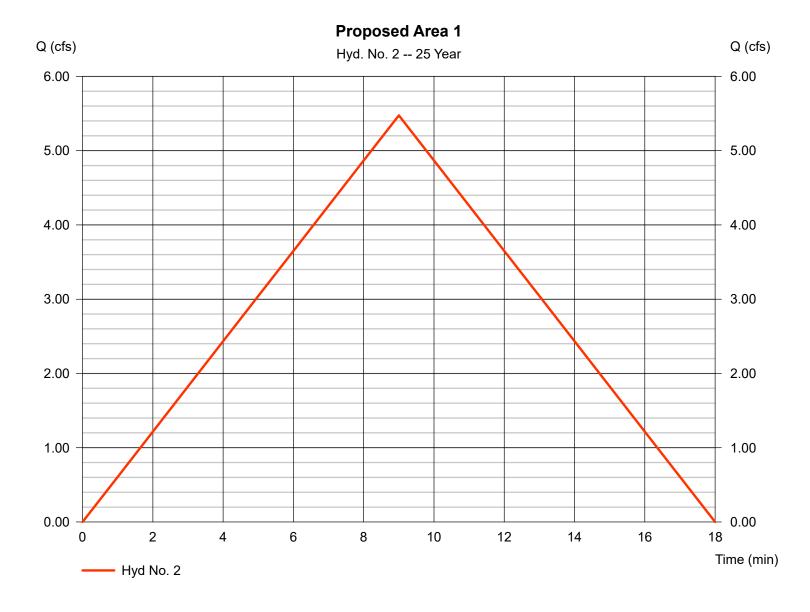
Proposed Area 1

Hydrograph type = Rational
Storm frequency = 25 yrs
Time interval = 1 min
Drainage area = 4.080 ac
Intensity = 6.710 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 5.475 cfs
Time to peak = 9 min
Hyd. volume = 2,956 cuft
Runoff coeff. = 0.2

Tc by TR55 = 9.00 min



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#### Hyd. No. 3

Water Quality Basin

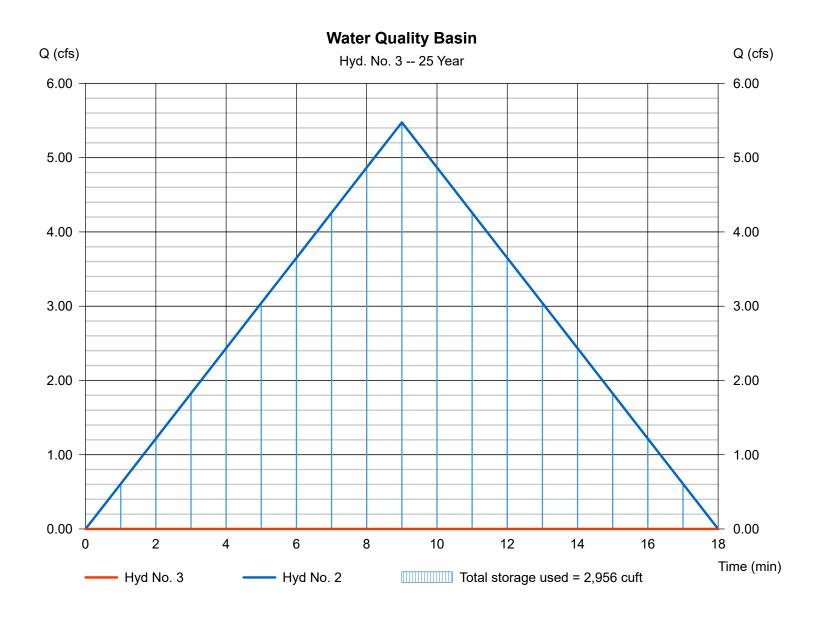
Hydrograph type = Reservoir Storm frequency = 25 yrs Time interval = 1 min

Inflow hyd. No. = 2 - Proposed Area 1

Reservoir name = Pond 1

Peak discharge = 0.000 cfs
Time to peak = n/a
Hyd. volume = 0 cuft
Max. Elevation = 204.74 ft
Max. Storage = 2,956 cuft

Storage Indication method used.



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#### Hyd. No. 4

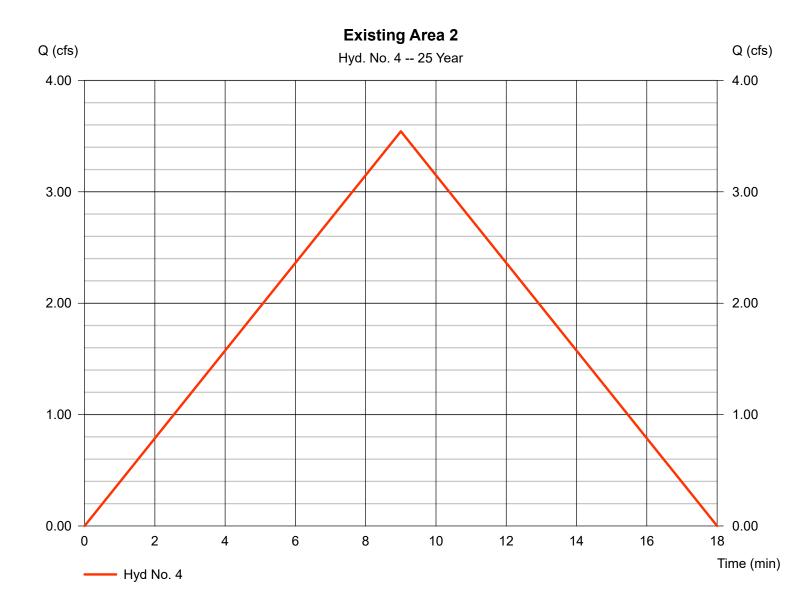
**Existing Area 2** 

Hydrograph type = Rational
Storm frequency = 25 yrs
Time interval = 1 min
Drainage area = 2.640 ac
Intensity = 6.710 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 3.543 cfs
Time to peak = 9 min
Hyd. volume = 1,913 cuft
Runoff coeff. = 0.2

Tc by TR55 = 9.00 minAsc/Rec limb fact = 1/1



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#### Hyd. No. 5

Proposed Area 2

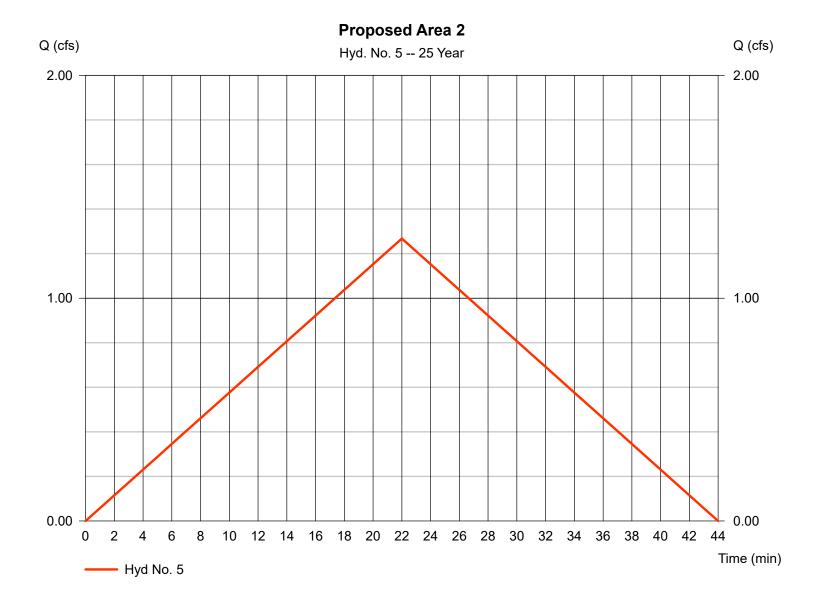
Hydrograph type = Rational
Storm frequency = 25 yrs
Time interval = 1 min
Drainage area = 1.540 ac

Intensity = 4.116 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 1.268 cfs
Time to peak = 22 min
Hyd. volume = 1,674 cuft
Runoff coeff. = 0.2

Tc by TR55 = 22.00 min



# Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

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Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	Rational	4.501	1	9	2,431				Existing Area 1
2	Rational	6.162	1	9	3,328				Proposed Area 1
3	Reservoir	0.000	1	n/a	0	2	204.83	3,328	Water Quality Basin
4	Rational	3.987	1	9	2,153				Existing Area 2
5	Rational	1.427	1	22	1,884				Proposed Area 2
GSI	D 69 - Drainaç	ge Calcul	ations - '	V1.gpw	Return P	eriod: 50 Y	ear	Friday, Apr	5, 2024

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#### Hyd. No. 1

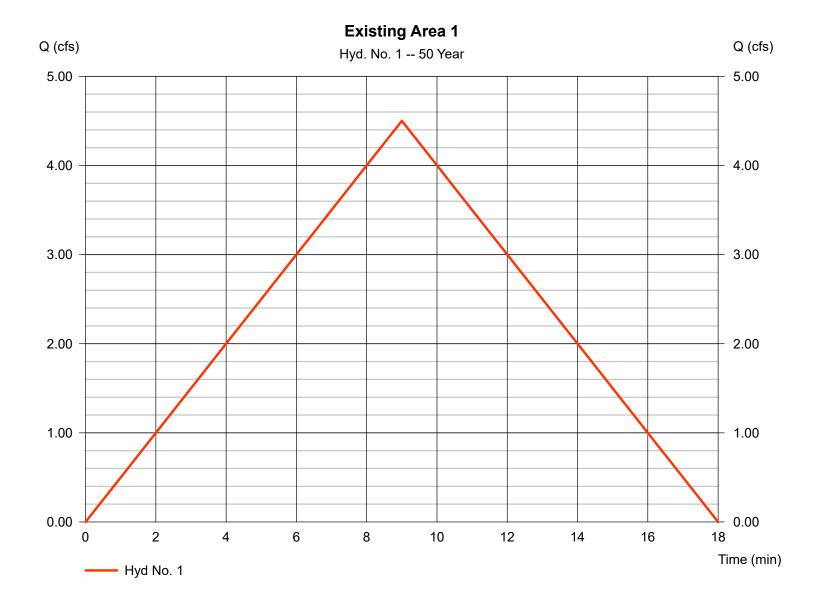
Existing Area 1

Hydrograph type = Rational
Storm frequency = 50 yrs
Time interval = 1 min
Drainage area = 2.980 ac
Intensity = 7.552 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 4.501 cfs
Time to peak = 9 min
Hyd. volume = 2,431 cuft
Runoff coeff. = 0.2

Tc by TR55 = 9.00 min



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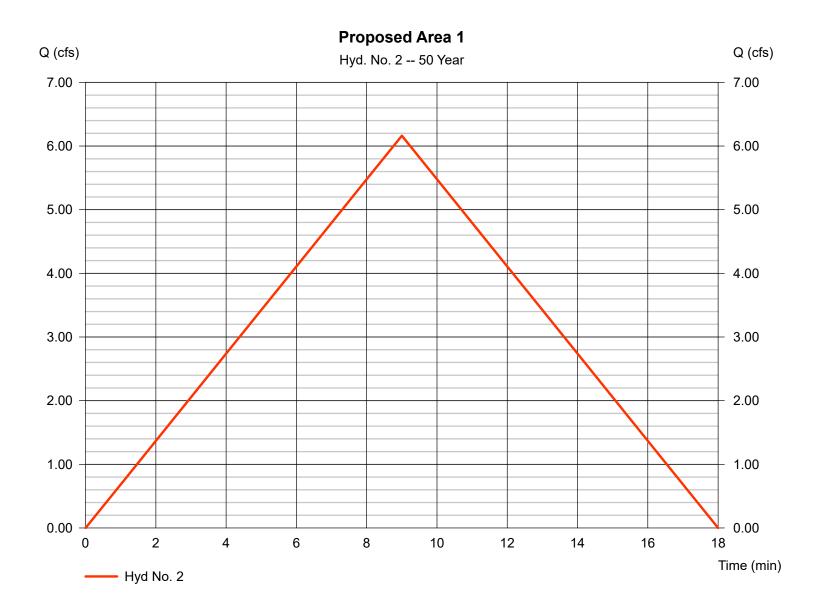
#### Hyd. No. 2

Proposed Area 1

Hydrograph type = Rational
Storm frequency = 50 yrs
Time interval = 1 min
Drainage area = 4.080 ac
Intensity = 7.552 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 6.162 cfs
Time to peak = 9 min
Hyd. volume = 3,328 cuft
Runoff coeff. = 0.2
Tc by TR55 = 9.00 min



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Friday, Apr 5, 2024

#### Hyd. No. 3

Water Quality Basin

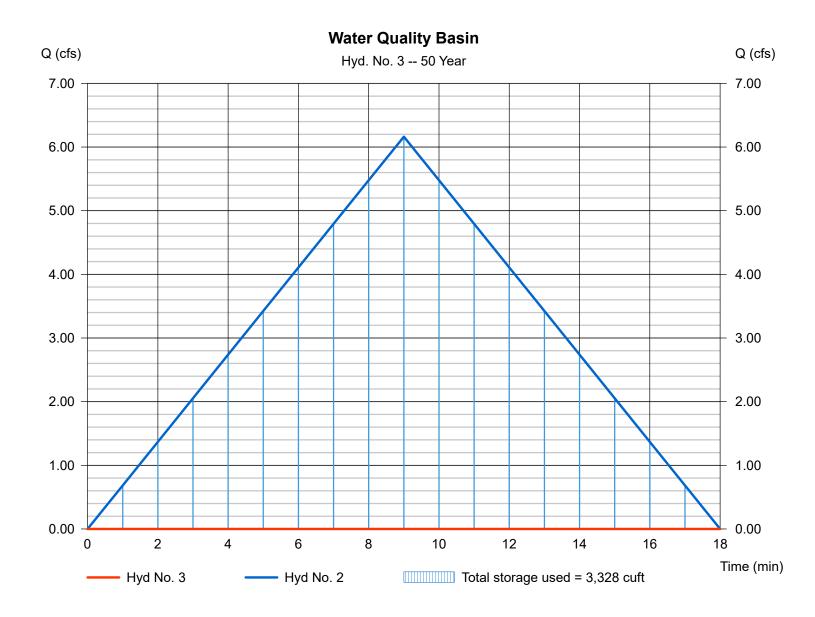
Hydrograph type = Reservoir Storm frequency = 50 yrs Time interval = 1 min

Inflow hyd. No. = 2 - Proposed Area 1

Reservoir name = Pond 1

Peak discharge = 0.000 cfs
Time to peak = n/a
Hyd. volume = 0 cuft
Max. Elevation = 204.83 ft
Max. Storage = 3,328 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Friday, Apr 5, 2024

#### Hyd. No. 4

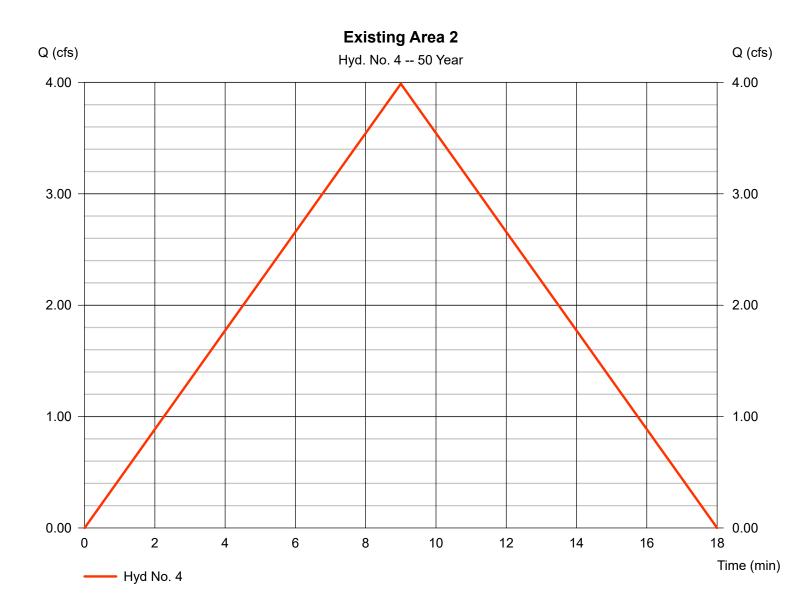
**Existing Area 2** 

Hydrograph type = Rational
Storm frequency = 50 yrs
Time interval = 1 min
Drainage area = 2.640 ac
Intensity = 7.552 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 3.987 cfs
Time to peak = 9 min
Hyd. volume = 2,153 cuft
Runoff coeff. = 0.2

Tc by TR55 = 9.00 minAsc/Rec limb fact = 1/1



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Friday, Apr 5, 2024

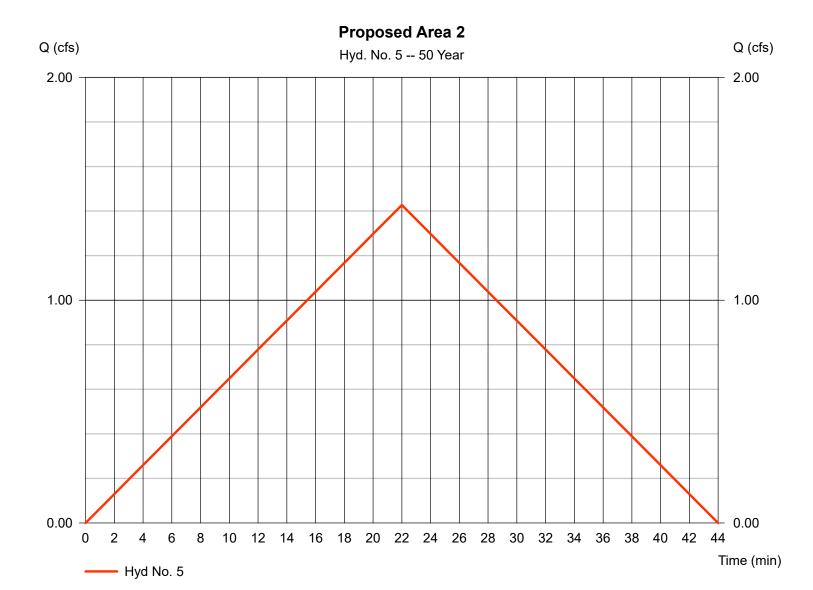
#### Hyd. No. 5

Proposed Area 2

Hydrograph type Peak discharge = 1.427 cfs= Rational Storm frequency Time to peak = 22 min = 50 yrsTime interval = 1 min Hyd. volume = 1,884 cuft Drainage area = 1.540 acRunoff coeff.

= 0.2Intensity = 4.633 in/hrTc by TR55 = 22.00 min

IDF Curve = GSD-60 NOAA.IDF Asc/Rec limb fact = 1/1



# Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

									, , , , , , , , , , , , , , , , , , ,	
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description	
1	Rational	5.038	1	9	2,721				Existing Area 1	
2	Rational	6.898	1	9	3,725				Proposed Area 1	
3	Reservoir	0.000	1	n/a	0	2	204.92	3,725	Water Quality Basin	
4	Rational	4.464	1	9	2,410				Existing Area 2	
5	Rational	1.597	1	22	2,109				Proposed Area 2	
GSI	GSD 69 - Drainage Calculations - V1.gpw				Return Period: 100 Year			Friday, Apr 5, 2024		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Friday, Apr 5, 2024

= 5.038 cfs

= 2,721 cuft

= 9 min

= 0.2

Peak discharge

Time to peak

Hyd. volume

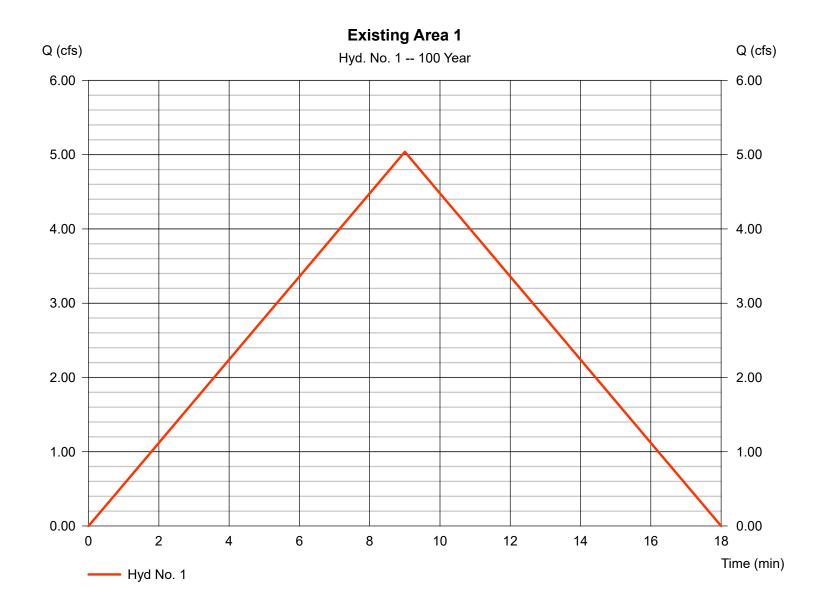
Runoff coeff.

#### Hyd. No. 1

**Existing Area 1** 

Hydrograph type = Rational
Storm frequency = 100 yrs
Time interval = 1 min
Drainage area = 2.980 ac
Intensity = 8.454 in/hr

Intensity = 8.454 in/hr Tc by TR55 = 9.00 min IDF Curve = GSD-60 NOAA.IDF Asc/Rec limb fact = 1/1



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Friday, Apr 5, 2024

#### Hyd. No. 2

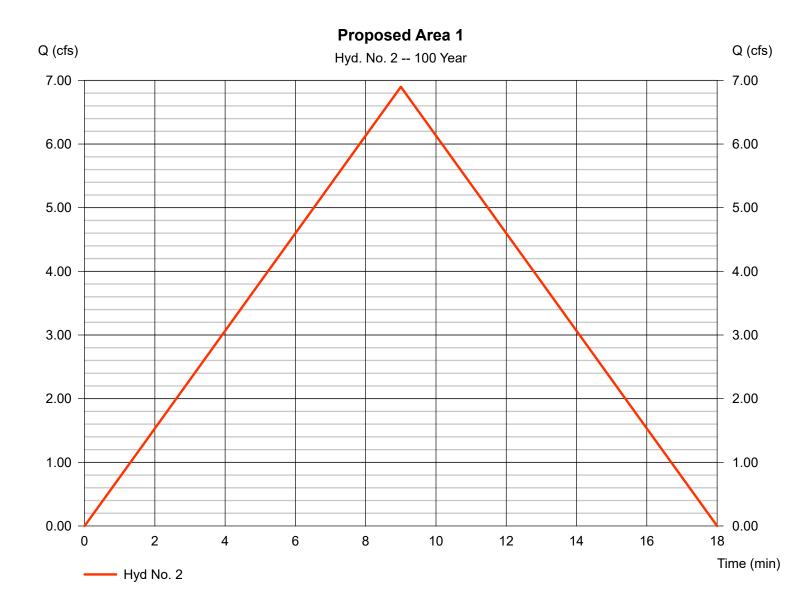
Proposed Area 1

Hydrograph type = Rational
Storm frequency = 100 yrs
Time interval = 1 min
Drainage area = 4.080 ac
Intensity = 8.454 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 6.898 cfs
Time to peak = 9 min
Hyd. volume = 3,725 cuft
Runoff coeff. = 0.2

Tc by TR55 = 9.00 minAsc/Rec limb fact = 1/1



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Friday, Apr 5, 2024

### Hyd. No. 3

Water Quality Basin

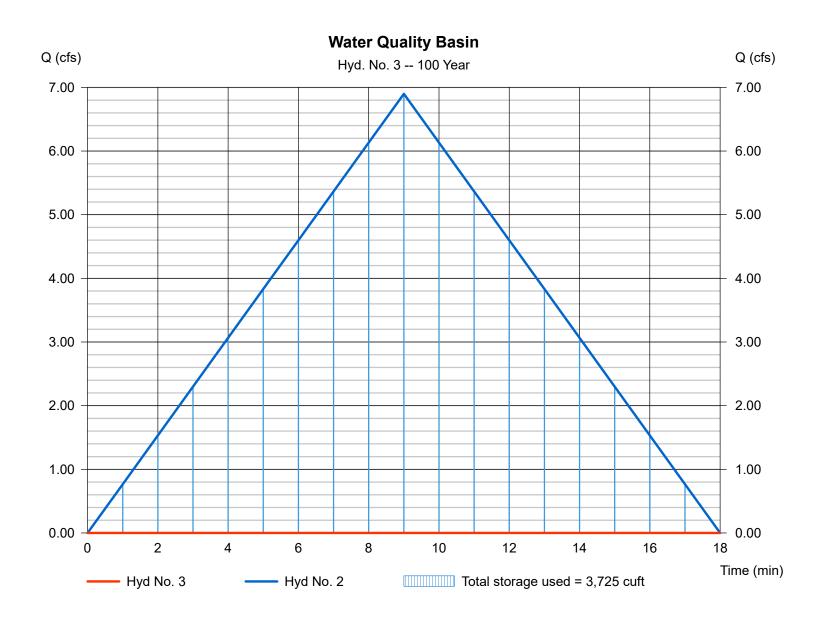
Hydrograph type = Reservoir Storm frequency = 100 yrs Time interval = 1 min

Inflow hyd. No. = 2 - Proposed Area 1

Reservoir name = Pond 1

Peak discharge = 0.000 cfs
Time to peak = n/a
Hyd. volume = 0 cuft
Max. Elevation = 204.92 ft
Max. Storage = 3,725 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Friday, Apr 5, 2024

#### Hyd. No. 4

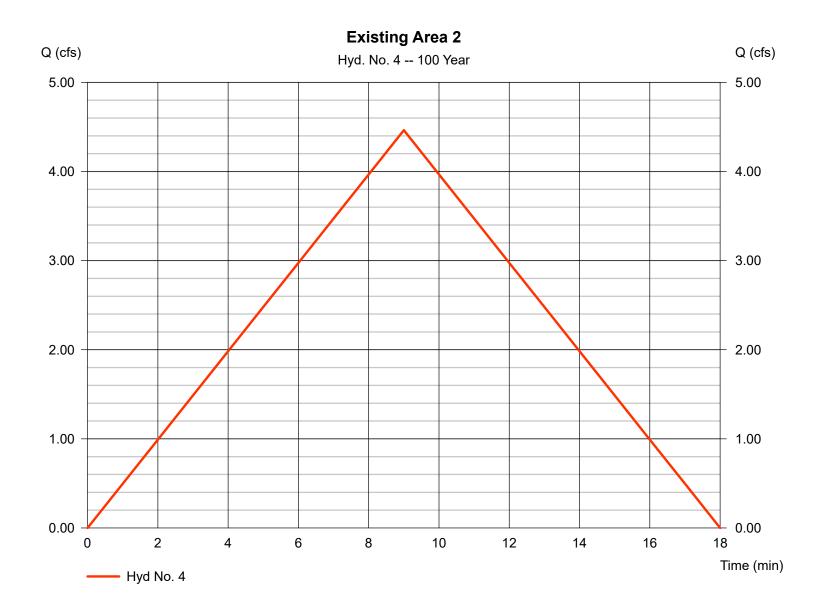
**Existing Area 2** 

Hydrograph type = Rational
Storm frequency = 100 yrs
Time interval = 1 min
Drainage area = 2.640 ac
Intensity = 8.454 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 4.464 cfs
Time to peak = 9 min
Hyd. volume = 2,410 cuft
Runoff coeff. = 0.2

Tc by TR55 = 9.00 minAsc/Rec limb fact = 1/1



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Friday, Apr 5, 2024

### Hyd. No. 5

Proposed Area 2

Hydrograph type = Rational
Storm frequency = 100 yrs
Time interval = 1 min
Drainage area = 1.540 ac

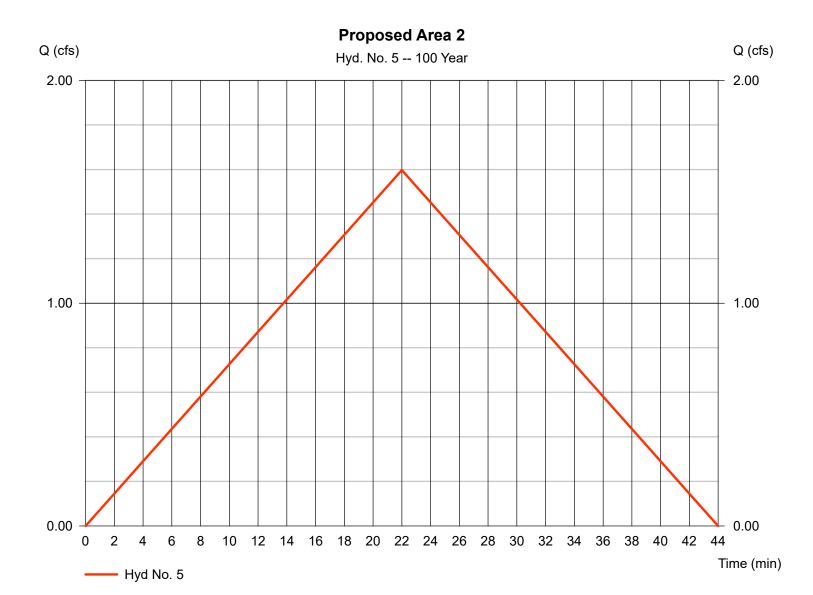
Intensity = 5.186 in/hr

IDF Curve = GSD-60 NOAA.IDF

Peak discharge = 1.597 cfs
Time to peak = 22 min
Hyd. volume = 2,109 cuft
Runoff coeff. = 0.2

Tc by TR55 = 22.00 min

Asc/Rec limb fact = 1/1





NOAA Atlas 14, Volume 10, Version 3 Location name: Oakdale, Connecticut, USA\* Latitude: 41.4238°, Longitude: -72.2074° Elevation: 217 ft\*\*

source: ESRI Maps
\*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

#### PF tabular

Durotion	Average recurrence interval (years)									
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	<b>4.06</b> (3.16-5.06)	<b>4.85</b> (3.77-6.06)	<b>6.14</b> (4.76-7.70)	<b>7.24</b> (5.58-9.12)	<b>8.72</b> (6.52-11.4)	<b>9.84</b> (7.20-13.1)	<b>11.0</b> (7.85-15.2)	<b>12.4</b> (8.33-17.3)	<b>14.3</b> (9.28-20.6)	<b>15.9</b> (10.1-23.3)
10-min	<b>2.87</b> (2.23-3.59)	<b>3.44</b> (2.67-4.30)	<b>4.36</b> (3.38-5.47)	<b>5.12</b> (3.94-6.45)	<b>6.18</b> (4.61-8.09)	<b>6.97</b> (5.10-9.29)	<b>7.81</b> (5.56-10.8)	<b>8.77</b> (5.90-12.3)	<b>10.1</b> (6.57-14.6)	<b>11.3</b> (7.13-16.5)
15-min	<b>2.25</b> (1.75-2.81)	<b>2.70</b> (2.10-3.37)	<b>3.42</b> (2.65-4.29)	<b>4.02</b> (3.10-5.06)	<b>4.85</b> (3.62-6.34)	<b>5.47</b> (4.00-7.29)	<b>6.12</b> (4.36-8.44)	<b>6.87</b> (4.62-9.60)	<b>7.95</b> (5.15-11.5)	<b>8.84</b> (5.60-13.0)
30-min	<b>1.58</b> (1.23-1.98)	<b>1.89</b> (1.47-2.37)	<b>2.40</b> (1.86-3.01)	<b>2.83</b> (2.18-3.56)	<b>3.41</b> (2.54-4.45)	<b>3.84</b> (2.81-5.12)	<b>4.30</b> (3.06-5.92)	<b>4.82</b> (3.25-6.74)	<b>5.57</b> (3.61-8.03)	<b>6.19</b> (3.92-9.07)
60-min	<b>1.02</b> (0.793-1.27)	<b>1.22</b> (0.948-1.52)	<b>1.55</b> (1.20-1.94)	<b>1.82</b> (1.40-2.29)	<b>2.19</b> (1.64-2.87)	<b>2.47</b> (1.81-3.30)	<b>2.77</b> (1.97-3.81)	<b>3.10</b> (2.09-4.34)	<b>3.59</b> (2.32-5.16)	<b>3.98</b> (2.52-5.84)
2-hr	<b>0.670</b> (0.525-0.832)	<b>0.800</b> (0.627-0.994)	<b>1.01</b> (0.790-1.26)	<b>1.19</b> (0.923-1.49)	<b>1.43</b> (1.08-1.86)	<b>1.61</b> (1.19-2.14)	<b>1.81</b> (1.30-2.47)	<b>2.03</b> (1.37-2.82)	<b>2.36</b> (1.54-3.37)	<b>2.64</b> (1.68-3.83)
3-hr	<b>0.519</b> (0.408-0.641)	<b>0.619</b> (0.486-0.765)	<b>0.782</b> (0.613-0.970)	<b>0.917</b> (0.715-1.14)	<b>1.10</b> (0.834-1.43)	<b>1.24</b> (0.920-1.64)	<b>1.39</b> (1.00-1.90)	<b>1.56</b> (1.06-2.16)	<b>1.82</b> (1.19-2.59)	<b>2.04</b> (1.30-2.94)
6-hr	<b>0.331</b> (0.263-0.406)	<b>0.394</b> (0.312-0.484)	<b>0.496</b> (0.392-0.611)	<b>0.581</b> (0.457-0.720)	<b>0.699</b> (0.531-0.898)	<b>0.786</b> (0.586-1.03)	<b>0.879</b> (0.637-1.19)	<b>0.989</b> (0.674-1.35)	<b>1.15</b> (0.754-1.62)	<b>1.29</b> (0.823-1.84)
12-hr	<b>0.204</b> (0.163-0.249)	<b>0.242</b> (0.193-0.296)	<b>0.305</b> (0.243-0.373)	<b>0.357</b> (0.283-0.439)	<b>0.429</b> (0.328-0.546)	<b>0.482</b> (0.361-0.626)	<b>0.539</b> (0.393-0.722)	<b>0.605</b> (0.415-0.819)	<b>0.703</b> (0.462-0.977)	<b>0.784</b> (0.503-1.11)
24-hr	<b>0.120</b> (0.097-0.145)	<b>0.143</b> (0.115-0.174)	<b>0.182</b> (0.146-0.221)	<b>0.214</b> (0.170-0.261)	<b>0.257</b> (0.198-0.326)	<b>0.290</b> (0.219-0.374)	<b>0.325</b> (0.238-0.432)	<b>0.366</b> (0.252-0.491)	<b>0.426</b> (0.282-0.587)	<b>0.477</b> (0.307-0.668
2-day	<b>0.067</b> (0.054-0.080)	<b>0.081</b> (0.066-0.097)	<b>0.104</b> (0.084-0.125)	<b>0.123</b> (0.099-0.149)	<b>0.149</b> (0.116-0.188)	<b>0.168</b> (0.128-0.216)	<b>0.189</b> (0.140-0.251)	<b>0.215</b> (0.148-0.285)	<b>0.253</b> (0.167-0.345)	<b>0.285</b> (0.184-0.395
3-day	<b>0.048</b> (0.039-0.058)	<b>0.058</b> (0.048-0.070)	<b>0.075</b> (0.061-0.090)	<b>0.089</b> (0.072-0.107)	<b>0.108</b> (0.084-0.135)	<b>0.122</b> (0.093-0.155)	<b>0.137</b> (0.102-0.180)	<b>0.155</b> (0.107-0.205)	<b>0.183</b> (0.121-0.248)	<b>0.207</b> (0.134-0.285
4-day	<b>0.039</b> (0.032-0.046)	<b>0.047</b> (0.038-0.056)	<b>0.060</b> (0.049-0.072)	<b>0.071</b> (0.057-0.085)	<b>0.086</b> (0.067-0.107)	<b>0.097</b> (0.074-0.123)	<b>0.109</b> (0.081-0.143)	<b>0.123</b> (0.085-0.162)	<b>0.145</b> (0.096-0.196)	<b>0.164</b> (0.106-0.224
7-day	<b>0.026</b> (0.022-0.031)	<b>0.031</b> (0.026-0.037)	<b>0.039</b> (0.032-0.047)	<b>0.046</b> (0.038-0.055)	<b>0.055</b> (0.044-0.069)	<b>0.062</b> (0.048-0.079)	<b>0.070</b> (0.052-0.091)	<b>0.079</b> (0.055-0.103)	<b>0.092</b> (0.061-0.123)	<b>0.103</b> (0.067-0.140
10-day	<b>0.021</b> (0.017-0.025)	<b>0.025</b> (0.020-0.030)	<b>0.031</b> (0.025-0.037)	<b>0.036</b> (0.029-0.043)	<b>0.043</b> (0.034-0.053)	<b>0.048</b> (0.037-0.060)	<b>0.053</b> (0.040-0.069)	<b>0.060</b> (0.042-0.077)	<b>0.069</b> (0.046-0.092)	<b>0.077</b> (0.050-0.103
20-day	<b>0.015</b> (0.012-0.018)	<b>0.017</b> (0.014-0.020)	<b>0.020</b> (0.017-0.024)	<b>0.023</b> (0.019-0.027)	<b>0.026</b> (0.021-0.032)	<b>0.029</b> (0.022-0.036)	<b>0.032</b> (0.024-0.040)	<b>0.035</b> (0.025-0.045)	<b>0.039</b> (0.026-0.052)	<b>0.042</b> (0.028-0.057
30-day	<b>0.012</b> (0.010-0.015)	<b>0.014</b> (0.011-0.016)	<b>0.016</b> (0.013-0.019)	<b>0.018</b> (0.015-0.021)	<b>0.020</b> (0.016-0.025)	<b>0.022</b> (0.017-0.027)	<b>0.024</b> (0.018-0.030)	<b>0.026</b> (0.018-0.033)	<b>0.029</b> (0.019-0.037)	<b>0.030</b> (0.020-0.040
45-day	<b>0.010</b> (0.009-0.012)	<b>0.011</b> (0.009-0.013)	<b>0.013</b> (0.011-0.015)	<b>0.014</b> (0.012-0.016)	<b>0.016</b> (0.012-0.019)	<b>0.017</b> (0.013-0.021)	<b>0.018</b> (0.014-0.023)	<b>0.020</b> (0.014-0.025)	<b>0.021</b> (0.014-0.028)	<b>0.022</b> (0.014-0.029
60-day	0.009	0.010	0.011	0.012	<b>0.013</b> (0.010-0.016)	0.014	0.015	0.016	0.017	0.018

<sup>&</sup>lt;sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

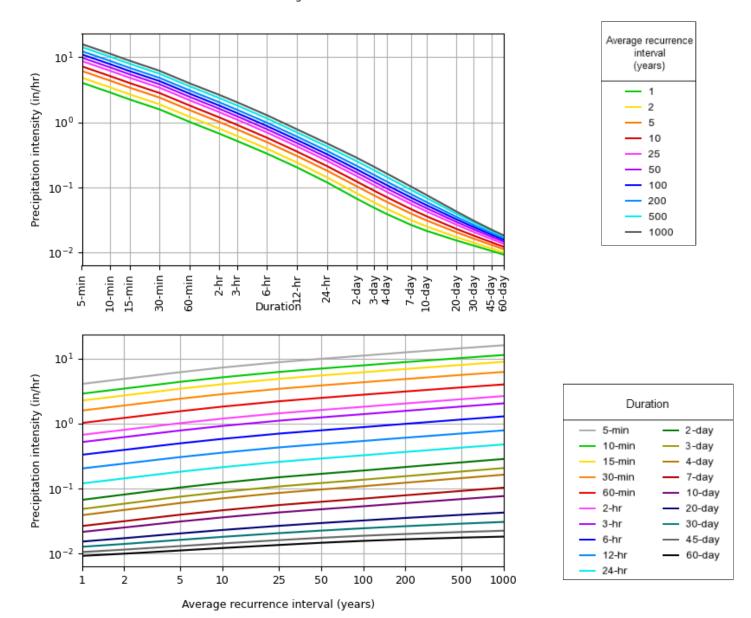
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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#### PF graphical

PDS-based intensity-duration-frequency (IDF) curves Latitude: 41.4238°, Longitude: -72.2074°



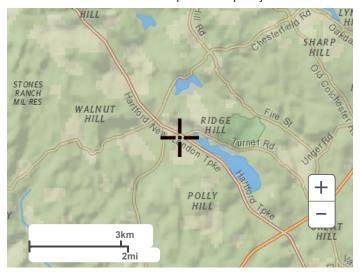
NOAA Atlas 14, Volume 10, Version 3

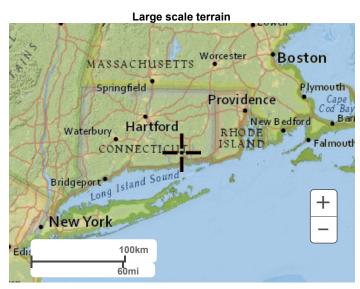
Created (GMT): Fri Apr 5 20:03:08 2024

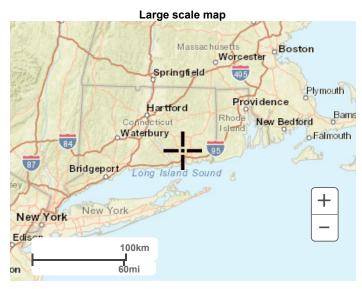
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#### Maps & aerials

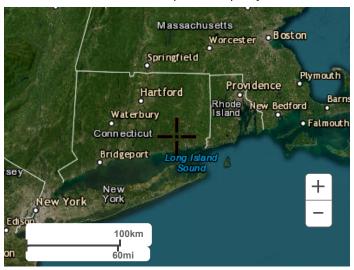
Small scale terrain







Large scale aerial



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US Department of Commerce

National Oceanic and Atmospheric Administration

National Weather Service
National Water Center

1325 East West Highway
Silver Spring, MD 20910

Questions?: https://doi.org/10.1001/html.

**Disclaimer** 



#### MAP LEGEND

#### Area of Interest (AOI)

Area of Interest (AOI)

#### Soils

Soil Map Unit Polygons



Soil Map Unit Points

#### Special Point Features

Blowout

Borrow Pit

Clay Spot

Closed Depression

Gravel Pit

Gravelly Spot

Landfill

Lava Flow

Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop

Saline Spot
Sandy Spot

Severely Eroded Spot

Sinkhole

Slide or Slip

Sodic Spot

Spoil Area

Stony Spot

Wery Stony Spot

Wet Spot

△ Other

Special Line Features

#### **Water Features**

Streams and Canals

#### Transportation

Rails

Interstate Highways

US Routes

Major Roads

Local Roads

#### Background

Aerial Photography

#### MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12.000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: State of Connecticut, Eastern Part Survey Area Data: Version 1, Sep 15, 2023

Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: Jun 14, 2022—Oct 6, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## **Map Unit Legend**

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
3	Ridgebury, Leicester, and Whitman soils, 0 to 8 percent slopes, extremely stony	0.3	5.0%
29A	Agawam fine sandy loam, 0 to 3 percent slopes	1.7	29.6%
84B	Paxton and Montauk fine sandy loams, 3 to 8 percent slopes	1.8	31.7%
84C	Paxton and Montauk fine sandy loams, 8 to 15 percent slopes	2.0	33.7%
Totals for Area of Interest	,	5.8	100.0%